

JOSH STEIN • Governor DEVDUTTA SANGVAI • Secretary DEBRA FARRINGTON • Deputy Secretary for Health KELLY KIMPLE • Acting Director, Division of Public Health

**Commission for Public Health** 

# MEMORANDUM

- TO: Interested Persons
- **FROM:** Virginia Niehaus, Rulemaking Coordinator, Commission for Public Health Director of Regulatory and Legal Affairs, Division of Public Health

**RE:** Notification of Proposed Rule Amendments: 15A NCAC 18E

Pursuant to G.S. 150B-21.2, this memorandum serves as the required notice to interested persons that the North Carolina Commission for Public Health (CPH) is proposing to amend rules 15A NCAC 18E .0102, .0105, .0202-.0204, .0206, .0207, .0301-.0303, .0401, .0402, .0504, .0506, .0508, .0601, .0701-.0703, .0801, .0805, .0901-.0909, .1101, .1202, .1203, .1205, .1301, .1303, .1305, .1306, .1401-.1405, and .1713. These rules are proposed for amendment to align with Session Laws 2023-77, 2023-63, s. 16, and 2024-49 s.4.10-4.47. CPH has submitted notice of its intent to amend these rules to the NC Office of Administrative Hearings (OAH).

In accordance with G.S. 150B-21.4, the NC Office of State Budget and Management (OSBM) was consulted on the proposed rules and determined that a fiscal note is not required, as the rules are required to be "substantively identical" to the session laws.

The notice of text that was published in today's edition of the NC Register is attached to this memorandum and may be found on OAH's website at <u>https://www.oah.nc.gov/rules-division/north-carolina-register</u>. The text of the proposed rules may be found on the CPH's website at <u>https://cph.dph.ncdhhs.gov/</u>.

A public hearing on these rules is scheduled for Tuesday, June 24, 2025 at 2:00 pm. The public hearing will be held by teleconference. You may participate in the public hearing by dialing 919-715-0769. No access code is required.

CPH is accepting public comments on the proposed rules from June 2, 2025 through August 1, 2025. You may submit comments by email to <u>cphcomment@ml.nc.gov</u> or by mail to Virginia Niehaus, CPH Rulemaking Coordinator, 1931 Mail Service Center, Raleigh, NC 27699-1931. Comments will also be accepted at the public hearing. No effective date is proposed, as the rules are subject to legislative review.

Should you have questions related to this memorandum or the proposed rules, please contact Ms. Trish Angoli, Environmental Engineer, On-Site Water Protection Branch, Environmental Health Section, Division of Public Health at 919-218-2580 or trish.angoli@dhhs.nc.gov.

Attachment

#### NC DEPARTMENT OF HEALTH AND HUMAN SERVICES • DIVISION OF PUBLIC HEALTH

LOCATION: 5605 Six Forks Road, Building 3, Raleigh, NC 27609 MAILING ADDRESS: 1931 Mail Service Center, Raleigh, NC 27699-1931 www.ncdhhs.gov • TEL: 919-707-5000 • FAX: 919-870-4829 cc: Dr. Ronald May, Chair, Commission for Public Health
 Dr. Kelly Kimple, Acting Director, Division of Public Health
 Mr. Larry Michael, Section Chief, Environmental Health, Division of Public Health
 Mr. Jon Fowlkes, Deputy Section Chief, Environmental Health, Division of Public Health
 Mr. Wilson Mize, On-Site Wastewater Protection Branch Head, Division of Public Health
 Ms. Trish Angoli, On-Site Wastewater Protection Branch, Division of Public Health
 Ms. Devon Collins, Deputy Director, Regulatory and Legal Affairs, Division of Public Health

### TITLE 15A – DEPARTMENT OF ENVIRONMENTAL QUALITY

*Notice* is hereby given in accordance with G.S. 150B-21.2 that the Commission for Public Health intends to amend the rules cited as 15A NCAC 18E .0102, .0105, .0202-.0204, .0206, .0207, .0301-.0303, .0401, .0402, .0504, .0506, .0508, .0601, .0701-.0703, .0801, .0805, .0901-.0909, .1101, .1202, .1203, .1205, .1301, .1303, .1305, .1306, .1401-.1405, and .1713.

Link to agency website pursuant to G.S. 150B-19.1(c): https://cph.dph.ncdhhs.gov/

Proposed Effective Date: Subject to Legislative Review

 Public Hearing:

 Date: June 24, 2025

 Time: 2:00 p.m.

 Location: This public hearing will be held by teleconference at (919) 715-0769 (no access code required).

**Reason for Proposed Action:** The Commission for Public Health proposes to amend the rules governing on-site wastewater treatment systems to align with Session Laws 2023-77, 2023-63, s.16, and 2024-49 s.4.10 - 4.47. These rule amendments are subject to legislative review.

**Comments may be submitted to:** Virginia Niehaus, CPH Rulemaking Coordinator, 1931 Mail Service Center, Raleigh, NC 27699-1931; email cphcomment@ml.nc.gov

Comment period ends: August 1, 2025

Rule(s) is automatically subject to legislative review: SL 2023-77, 2023-63, s.16, 2024-49 s.4.10 - 4.47

Fiscal impact. Does any rule or combination of rules in this notice create an economic impact? Check all that apply.

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State funds affected Local funds affected Substantial economic impact (>= \$1,000,000)

Approved by OSBM

No fiscal note required

# **CHAPTER 18 - ENVIRONMENTAL HEALTH**

# SUBCHAPTER 18E – WASTEWATER TREATMENT AND DISPERSAL SYSTEMS

# **SECTION .0100 – GENERAL**

# 15A NCAC 18E .0102 APPLICABILITY

(a) The rules of this Subchapter shall not apply to <u>any</u> wastewater systems system for which an IP, CA, OP, NOI, ATO, certificate of <u>completion</u>, or an equivalent approval has been issued prior to January 1, 2024, in use which are not malfunctioning as described in <del>Rule .1303(a)(2)</del> of this Subchapter, unless the DDF or wastewater strength <u>increases</u>. increases or unless otherwise specified in this <del>Subchapter.</del> Wastewater systems that are malfunctioning in accordance with Rule .1303(a)(2) of this Subchapter shall adhere to the rules of this Subchapter. The provisions of Rule .0206 of this Subchapter shall apply to any wastewater system for which an OP, ATO, certificate of completion, or an equivalent approval has been issued prior to January 1, 2024.

(b) The rules of this Subchapter shall not apply to IPs and CAs issued prior to the effective date of this Rule. For purposes of this Rule, wastewater strength shall not be determined to be increased unless the facility is a commercial facility or becomes a commercial facility and the wastewater strength is determined to meet the definition of HSE in accordance with Rule .0402 of this Subchapter.

(c) Prior to any increase in DDF or wastewater strength for an existing facility, the owner shall submit an application in accordance with Rule .0202 of this Subchapter.

(d) Wastewater systems permitted prior to January 1, 2024, on or after July 1, 1977, shall comply with the setback requirements in place at the time the IP, CA, OP, NOI, ATO, certificate of completion, or equivalent was issued. this Subchapter.

(e) Wastewater systems installed prior to July 1, 1977, and existing wastewater systems for which a permit cannot be found are not considered to meet the conditions of Rules .0302(c)(5) or .0302(c)(6) of this Subchapter.

(d)(f) Notwithstanding Paragraph (a) of this Rule, all wastewater systems shall comply with Section .1300 of this Subchapter.

(g) When a wastewater system that was installed on or after July 1, 1977, is expanded, modified, or repaired, and the wastewater strength is not increasing, any existing wastewater system components that are not crushed, broken, damaged, or otherwise rendered unusable or ineffective so that the component will not function as designed shall not be required to meet the requirements of this Subchapter. All components shall comply with the setback requirements in this Subchapter.

(h) When a wastewater system that was installed prior to July 1, 1977, is expanded, modified, or repaired, and the wastewater strength is not increasing, any existing wastewater system components that are not crushed, broken, damaged, or otherwise rendered unusable or ineffective so that the component will not function as designed shall not be required to meet the requirements of this Subchapter except as follows:

- (1) Setbacks to drinking water wells shall not be reduced; and
- (2) Setbacks to surface water bodies specified in Rule .0601 of this Subchapter shall not be reduced by more than 50 percent.

(i) Existing wastewater systems for which no permit can be found and with no evidence that the wastewater system was installed in violation of G.S. 130A, Article 11 and the rules in effect at the time of installation shall be considered to have an OP or its equivalent in accordance with this Rule.

History Note: Authority G.S. 130A-335(e); <u>S.L. 2023-77, s.1; S.L, 2024-49, s.4.10;</u> Eff. January 1, 2024.

#### 15A NCAC 18E .0105 DEFINITIONS

In addition to the definitions set forth in G.S. 130A-334, the following shall apply to the Rules in this Subchapter:

- (1) "Accessory dwelling unit" means a secondary dwelling unit established in conjunction with and clearly subordinate to a primary dwelling unit, whether a part of the same structure as the primary dwelling unit or a detached dwelling unit on the same lot. The DDF for an accessory dwelling unit shall be based on 120 gpd per bedroom, or 60 gpd per person when occupancy exceeds two persons per bedroom. The accessory dwelling unit shall not be considered a separate dwelling unit when sizing a septic tank for a primary dwelling unit and an accessory dwelling unit. When the primary dwelling unit and accessory dwelling unit are under common ownership, a multi-party agreement is not required.
- (1)(2) "Aggregate" means naturally occurring inorganic material of a specific size or grade. An example of aggregate is clean, washed gravel, or crushed stone that is graded or sized in accordance with size numbers 4, 5, or 6 of ASTM D448.
- (2)(3) "Apparent Cation Exchange Capacity" means the sum of exchangeable bases plus total soil acidity at a pH of 7.0. ACEC is expressed in milliequivalents per 100 grams (meq/100g) of soil or centimoles per kilogram (cmol/kg) of soil. The soil ACEC is calculated by determining the ACEC using the neutral normal ammonium acetate method, pH of 7.0 neutral normal, dividing by the percent clay as determined by particle size distribution using the pipette method, and then multiplying by 100, as described in USDA-NRCS Soil Survey Laboratory Information Manual, Soil Survey Investigations Report No. 45 and Kellogg Soil Survey Laboratory Methods Manual, Soil Survey Investigation Report No. 42, page 229, or EPA Method 9080.
- (3)(4) "Applicant" means the individual who submits an application to the LHD for an IP, CA, OP, <u>NOI, ATO</u>, or existing system authorization.
- (4)(5) "Approved" means that which the Department or LHD has determined is in accordance with this Subchapter and G.S. 130A, Article 11.
- (5)(6) "Artificial drainage" means any man-made structure or device designed to overcome a SWC or intercept lateral flowing ground or surface water. Artificial drainage systems include groundwater lowering systems, interceptor drains, foundation drains with cuts greater than two feet, and surface water diversions.
- (6)(7) "Authorized agent" means a person who has been authorized by the Department in accordance with G.S. 130A, Article 4 and 15A NCAC 01O .0100 to permit wastewater systems.
- (7)(8) "Authorized designer" means a service provider authorized by the manufacturer who creates plans for the installation, expansion, or repair of a proprietary wastewater system.
- (8)(9) "Authorized On-Site Wastewater Evaluator" means a person licensed in accordance with G.S. 90A, Article 5 and meeting the certification requirements in G.S. 130A-336.2(a) and 21 NCAC 39.
- (9)(10) "Backfill" means the soil that is placed in a trench or bed that surrounds or is on top of the dispersal media within the excavation up to the naturally occurring soil surface.
- (10)(11) "Bed" means an excavation with a width greater than three feet containing dispersal media and one or more laterals.
- (11)(12) "Bedroom" means any room defined as a sleeping room in the North Carolina Building Code.
- (12)(13) "Building drain" means the lowest piping of a drainage system that receives the discharge from waste pipes inside the design unit and extends to 10 ft beyond the walls of the building or five feet for a building with a foundation and conveys the sewage to a building sewer.
- (13)(14) "Building sewer" means the part of a drainage system that extends from the end of the building drain and conveys the discharge to a wastewater system.
- (14)(15) "Certified Inspector" means a person authorized to inspect a wastewater system in accordance with G.S. 90A, Article 5, and applicable rules of the North Carolina On-Site Wastewater Contractors and Inspectors Certification Board.
- (15)(16) "Clod" means a compact, coherent, mass of soil produced by digging, plowing, or other human land manipulation.
- (16)(17) "Coastal region" means Beaufort, Bertie, Bladen, Brunswick, Camden, Carteret, Chowan, Columbus, Craven, Cumberland, Currituck, Dare, Duplin, Edgecombe, Gates, Greene, Halifax, Harnett, Hertford, Hoke, Hyde, Johnston, Jones, Lenoir, Martin, New Hanover, Northampton, Onslow, Pamlico, Pender, Pasquotank, Perquimans, Pitt, Richmond, Robeson, Sampson, Scotland, Tyrrell, Washington, Wayne, and Wilson counties.

- (17)(18) "Collection sewer" means gravity flow pipelines, force mains, effluent supply lines, manholes, lift stations, and all appurtenances used for conveying wastes from the building drain or sewer to and within a wastewater system. A collection system is a collection sewer.
- (18)(19) "Complete data set" means analytical results for all required influent and effluent constituents as specified in the effluent standard for a specific site on a specific date. A data set may include other constituents specified in an RWTS or PIA Approval, permit, or other document.
- (19)(20) "Component" means a part of a wastewater system. The component may be any part of the wastewater system, such as a collection sewer, pretreatment, dispersal field, etc.
- (20)(21) "Composite sample" means commingled individual samples collected from the same point at different times. Samples may be of equal volume or may be proportional to the flow at time of sampling.
- (21)(22) "Control system" means either conventional or accepted systems that are surveyed as part of a survey protocol identified in Rule .1706 of this Subchapter.
- (22)(23) "Cover" means the soil that is placed at or above the naturally occurring soil surface to cover the wastewater system.
- (23)(24) "Demand dosing" means a configuration in which a specific volume of effluent is delivered to a component based upon patterns of wastewater generation from the source and liquid level detection device settings.
- (24)(25) "Department" means the North Carolina Department of Health and Human Services, as defined in G.S. 130A-334(1f). The mailing address for the Department is as follows: NCDHHS, Division of Public Health, On-Site Water Protection Branch, 1642 Mail Service Center, Raleigh, North Carolina 27699-1642.
- (25)(26) "Design daily flow" means the unadjusted quantity of wastewater a facility is projected to produce in a 24-hour period upon which wastewater system sizing and design are based as determined in Section .0400 of this Subchapter.
- (26)(27) "Design unit" means a discrete connection such as an individual dwelling unit, place of business, or place of public assembly on which wastewater DDF is based. Multiple design units may comprise a facility.
- (27)(28) "Dispersal field" means the physical location where final treatment and dispersal of effluent occurs in the soil.
- (28)(29) "Dispersal media" means the media used to provide void space through which effluent flows and may be stored prior to infiltration, such as washed gravel or crushed stone, products referenced in Section .0900 of this Subchapter, products approved pursuant to Section .1700 of this Subchapter, etc.
- (29)(30) "Dispersal system" means the dispersal field and associated components that distribute effluent to and within the dispersal field. This includes a pump, pump tank, pressure manifold, distribution box, drip box, lateral, dispersal media, etc.
- (30)(31) "Dose volume" means an amount of effluent delivered during a dosing event as determined by the liquid level detection device settings in a demand dosing system or by a timer in a timed dosing system.
- (31)(32) "Dwelling unit" means any room or group of rooms located within a structure and forming a single, habitable unit with facilities which are used or intended to be used for living, sleeping, bathing, toilet usage, cooking, and eating.
- (32)(33) "Effluent" means the liquid discharge from a pretreatment process, component, or system.
- (33)(34) "Facility" means one or more design units located on a single or multiple lot(s) or tract(s) of land and served by a wastewater system comprised of one or more wastewater systems.
- (34)(35) "Finished grade" means the final elevation of the land over the wastewater system after installation.
- (35)(36) "Flow equalization" means a system configuration that includes sufficient storage capacity to allow for uniform flow to a subsequent component despite variable flow from the source.
- (36)(37) "Full kitchen" means the appliances meet the requirements of North Carolina Food Code, Chapters 4-1 and 4-2. <u>a</u> kitchen that contains either domestic or commercial equipment and is used for cooking or preparing foods onsite.
- (37)(38) "Grab sample" means a discrete sample collected at a specific time and location.
- (38)(39) "Grease tank" means the tank located outside the facility that is used to reduce the amount of grease discharged to a wastewater system.
- (39)(40) "Grease trap" means a device used inside the facility to reduce the amount of grease discharged to a wastewater system.
- (40)(41) "Gravity distribution" means gravity flow of effluent to and within each lateral.
- (41)(42) "Groundwater lowering system" means a type of artificial drainage system designed to lower the water table by gravity or, in conjunction with a pump, to maintain the vertical separation beneath a dispersal field.
- (42)(43) "Horizon" means a layer of soil, parallel to the surface that has distinct physical, chemical, and biological properties or characteristics such as color, structure, texture, consistence, kinds and number of organisms present, degree of acidity or alkalinity, etc., resulting from soil forming processes.
- (43)(44) "Infiltrative surface" means the designated interface where effluent moves from dispersal media or a distribution device into treatment media, naturally occurring soil, or fill.
- (44)(45) "Influent" means the sewage discharged to a pretreatment component.
- (45)(46) "Installer" means a person authorized to construct, install, or repair a wastewater system in accordance with G.S. 90A, Article 5 and applicable rules of the North Carolina On-Site Wastewater Contractors and Inspectors Certification Board.
- (46)(47) "Interceptor drain" means a type of artificial drainage designed to intercept and divert lateral moving groundwater or perched water away from the dispersal field or other system component to an effective outlet.
- (48) "Intermittent stream" means a well-defined channel that contains water for only part of the year, typically during winter and spring when the aquatic bed is below the perched or seasonal high-water table. The flow of an intermittent stream may be supplemented by stormwater runoff.

(47)(49) "Invert" means the lowest elevation of the internal cross-section of a pipe, fitting, or component.

- (48)(50) "Jurisdictional wetland" means an area subject to the regulatory jurisdiction of the U.S. Army Corps of Engineers or DEQ.
- (49)(51) "Ksat" or saturated hydraulic conductivity, means the rate of water flow through a unit cross sectional area of soil under saturated conditions. In-situ Ksat is measured in the field using clean water. Results of in-situ Ksat are used to simulate movement of effluent through the soil and may be used to field verify LTAR.
- (50)(52) "Lateral water movement" means the movement of subsurface water downslope often associated with a less permeable horizon. Lateral water movement can be observed in a bore hole, excavation, or monitoring well on sloping sites.
- (51)(53) "Lateral" means any pipe, tubing, or other device used to convey and distribute effluent in a dispersal field.
- (52)(54) "Limiting condition" means soil conditions or site features that determine wastewater system design options. Soil conditions are morphology, depth, restrictive horizons, soil wetness, or organic matter content. Site features are topography, slope, landscape position, or available space.
- (53)(55) "Lithochromic feature" means soil mottle or matrix associated with variations of color due to weathering of parent materials.
- (54)(56) "Long Term Acceptance Rate" means the rate of effluent absorption by the soil, existing fill, or saprolite in a wastewater system after long-term use. The LTAR, in units of gpd/ft<sup>2</sup>, is assigned based upon soil textural class, structure, consistence, depth, percent coarse rock, landscape position, topography, and system type, and is used to determine the dispersal field sizing requirements, in accordance with applicable rules of this Subchapter.
- (55)(57) "Local health department" means any county, district, or other health department authorized to be organized under the General Statutes of North Carolina.
- (56)(58) "Management Entity" means the person, entity, company, or firm designated by the owner of the wastewater system who has primary responsibility for the operation of a wastewater system in accordance with this Subchapter, G.S. 90A, Article 3, and applicable rules of the Water Pollution Control System Operators Certification Commission. The Management Entity may be the owner, a public Management Entity, a certified operator, a management company, or an entity that employs certified operators. The Management Entity is or employs the operator in responsible charge for the wastewater system.
- (57)(59) "Mass loading" means the total mass of one or more organic or inorganic effluent constituents delivered to the wastewater system over a specified period. It is computed by multiplying the total volume of flow during the specified period by the flow-weighted average constituent concentration in the same period. Units of measurement are pounds per day.
- (58)(60) "Matrix" means a volume of soil equivalent to 50 percent or greater of the total volume of a horizon.
- (59)(61) "Mean high-water mark" means, for coastal waters having six inches or more lunar tidal influence, the average height of the high-water over a 19-year period as may be ascertained from National Ocean Survey, U.S. Army Corps of Engineers tide stations data, or as otherwise determined under the provisions of the Coastal Area Management Act. The highest high-water mark as reported by the three agencies shall be applied.
- (60)(62) "Media" means a solid material that can be described by shape, dimensions, surface area, void space, and application.
- (61)(63) "Media filter" means a device that uses materials designed to treat effluent by reducing BOD<sub>5</sub> and removing TSS in an unsaturated environment. Biological treatment is facilitated via microbial growth on the surface of the treatment media.
- (62)(64) "Mottle" means subordinate color of a differing Munsell color system notation in a soil horizon.
- (63)(65) "Mountain region" means Alleghany, Ashe, Avery, Buncombe, Cherokee, Clay, Graham, Haywood, Henderson, Jackson, Macon, Madison, McDowell, Mitchell, Swain, Transylvania, Watauga, and Yancey counties.
- (64)(66) "Naturally occurring soil" means soil formed in place due to natural formation processes that is unaltered by filling, removal, or other artificial modification other than tillage.
- (65)(67) "NEMA 4X" means an enclosure for an electrical control panel or junction box that meets standards for protection of equipment due to the ingress of water, including rain and hose-directed water, and an additional level of protection against corrosion, as set forth in NEMA Standard 250.
- (66)(68) "NSF/ANSI 40 systems" means individual RWTS that are approved and listed in accordance with the standards adopted by NSF International for Class I residential wastewater treatment systems under NSF/ANSI Standard 40 and approved for use in accordance with G.S. 130A-342 and the Rules of this Subchapter.
- (67)(69) "Non-ground absorption system" means a system for waste treatment designed not to discharge to the soil, land surface, or surface waters, including approved vault privies, incinerating toilets, mechanical toilets, composting toilets, chemical toilets, and recycling systems.
- (68)(70) "Normal water level" means the term as defined in 15A NCAC 02B .0610(28). water level within a pond, lake, or other type of impoundment, natural or man-made, at the elevation of the outlet structure or spillway, such as the elevation of the permanent pool.
- (69)(71) "Off-site system" means a wastewater system where any system component is located on property other than the lot where the facility is located.

- (70)(72) "Ordinary high-water mark" means the line on the shore established by the fluctuations of water and indicated by physical characteristics such as: a natural line impressed on the bank; shelving; changes in the character of soil; destruction of terrestrial vegetation; or the presence of litter and debris.
- (71)(73) "Organic soils" means those organic mucks and peats consisting of more than 20 percent organic matter, by dry weight, and greater than or equal to 18 inches in thickness.
- (72)(74) "Owner" means a person holding legal title to the facility, wastewater system, or property or his or her representative. The owner's representative is a person who holds power of attorney to act on an owner's behalf or an agent designated by letter or contract to act on the owner's behalf.
- (73)(75) "Parallel distribution" means the distribution of effluent that proportionally loads multiple sections of a dispersal field at one time.
- (74)(76) "Parent material" means the mineral and organic matter that is in its present position through rock decomposition or deposition by water, wind, or gravity.
- (75)(77) "Ped" means a unit of soil structure, such as blocky, granular, prismatic, or platy formed by natural processes.
- (76)(78) "Perched water table" means a zone of saturation held above the main groundwater body by a less permeable layer, impermeable rock, or sediment, which may or may not exhibit redoximorphic features.
- (79) "Perennial stream" means a well-defined channel that contains water year-round during a year of normal rainfall with the aquatic bed located below the perched or seasonal high water table for most of the year. Groundwater is the primary source of water for a perennial stream, but perennial streams may also carry stormwater runoff.
- (77)(80) "Person" means any individual, firm, association, organization, partnership, business trust, corporation, company, or unit of local government.
- (78)(81) "Piedmont region" means Alamance, Alexander, Anson, Burke, Cabarrus, Caldwell, Caswell, Catawba, Chatham, Cleveland, Davidson, Davie, Durham, Forsyth, Franklin, Gaston, Granville, Guilford, Iredell, Lee, Lincoln, Mecklenburg, Montgomery, Moore, Nash, Orange, Person, Polk, Randolph, Rockingham, Rowan, Rutherford, Stanly, Stokes, Surry, Union, Vance, Wake, Warren, Wilkes, and Yadkin counties.
- (79)(82) "Pressure dispersal" means a system utilizing an effluent pump or siphon to distribute effluent uniformly to the infiltrative surface in the dispersal field through a pressurized pipe network.
- (80)(83) "Pressure dosed gravity distribution" means pressure delivery of effluent to a manifold, distribution box, or other splitter with subsequent gravity distribution within one or more laterals to the infiltrative surface.
- (81)(84) "Public management entity" means a public entity legally authorized to operate and maintain wastewater systems, including a city pursuant to G.S. 160A, Article 16, a county pursuant to G.S. 153A, Article 15, an interlocal contract pursuant to G.S. 160A, Article 20, a joint management agency pursuant to G.S. 160A, Article 20, a county service district pursuant to G.S. 153A, Article 16, a county water and sewer district pursuant to G.S. 162A, Article 6, a sanitary district pursuant to G.S. 130A, Article 2, Part 2, a water and sewer authority pursuant to G.S. 162A, Article 1, a metropolitan water district pursuant to G.S. 162A, Article 4, a metropolitan sewerage district pursuant to G.S. 130A, Article 5A, a public utility pursuant to G.S. 62, Article 1, a county or district health department pursuant to G.S. 130A, Article 2, or any other public entity legally authorized to operate and maintain wastewater systems.
- (82)(85) "Raw sewage lift stations" means a dosing system that is designed to move untreated sewage from a lower elevation to a higher elevation. Raw sewage lift stations are installed prior to any wastewater treatment.
- (83)(86) "RCW systems" means advanced pretreatment systems approved by the Department in accordance with Section .1700 of this Subchapter that meet RCW effluent standards in Rule .1002 of this Subchapter.
- $\frac{(84)(87)}{(87)}$  "Redoximorphic features" means a color pattern of a horizon due to a depletion or concentration of pigment compared to the matrix color, formed by oxidation and reduction of Fe coupled with its removal, translocation, or accrual, or a soil matrix color controlled by the presence of Fe<sup>+2</sup>. Redox depletions are a type of redoximorphic feature.
- (85)(88) "Repair area" means an area that has been classified suitable consistent with the Rules in this Subchapter that is reserved for the extension, alteration, wastewater system relocation, or replacement of part or all of the initial wastewater system. The repair area shall be available to be used in the event of a malfunction or if a wastewater system is partially or totally destroyed.
- (86)(89) "Residential Wastewater Treatment Systems" means approved individual advanced pretreatment systems that are covered under standards of NSF International, in accordance with G.S. 130A-342 and applicable Rules in this Subchapter.
- (87)(90) "Restrictive horizon" means a soil horizon that is capable of perching groundwater or effluent and that is brittle and strongly compacted or strongly cemented with iron, aluminum, silica, organic matter, or other compounds. Restrictive horizons may occur as fragipans, iron pans, or organic pans, and are recognized by their resistance in excavation or in using a soil auger.
- (88)(91) "Rock" means the body of consolidated or partially consolidated material composed of minerals at or below the land surface. Rock includes bedrock and partially weathered rock that is hard and cannot be dug with hand tools. The upper boundary of rock is saprolite, soil, or the land surface.
- (89)(92) "Saprolite" means the body of porous material formed in place by weathering of rock that has a massive, rockcontrolled structure and retains the arrangement of minerals of its parent rock in a minimum of 50 percent of its volume. Saprolite can be dug with hand tools. The lower limit of saprolite is rock and its upper limit is soil or the land surface.

- (90)(93) "Septic tank" means a structurally sound, water-tight, covered receptacle, approved in accordance with Section .1400 of this Subchapter. A septic tank is designed for primary treatment of wastewater and is constructed to:
  - (a) receive the discharge of wastewater from a building;
  - (b) separate settleable and floating solids from the liquid;
  - (c) digest organic matter by anaerobic bacterial action;
  - (d) store digested solids through a period of detention; and
  - (e) allow effluent to discharge for additional treatment and final dispersal.
- (91)(94) "Septic tank effluent pump" means a collection system that uses a septic tank to separate solids and incorporates a pump vault, pump, and associated devices to convey effluent under pressure to a subsequent component.
- (92)(95) "Sequential distribution" means the distribution method in which effluent is loaded into one trench and fills it to a predetermined level before passing through a drop box or relief device to the succeeding trench at a lower elevation. All trenches are fed from the same side.
- (93)(96) "Setback" means the minimum horizontal separation distance between the wastewater system and features listed in Section .0600 of this Subchapter.
- (94)(97) "Settling tank" means a septic tank designed for use in conjunction with a RWTS. A settling tank is not required to meet the design requirements of a septic tank.
- (95)(98) "Serial distribution" means the distribution method in which effluent is loaded into one trench and fills it to a predetermined level before passing through a pipe to the succeeding trench at the same or a lower elevation.
- (96)(99) "Site" means the area in which the wastewater system is located, including the repair area.
- (97)(100)"Soil" means the naturally occurring body of unconsolidated mineral and organic materials on the land surface. Soil is composed of sand-, silt-, and clay-sized particles that are mixed with varying amounts of larger fragments and some organic material. Soil contains less than 50 percent of its volume as rock, saprolite, or coarse-earth fraction. The coarse-earth fraction are mineral particles greater than 2.0 millimeters. The upper limit of the soil is the land surface, and its lower limit is rock, saprolite, or other parent materials.
- (98)(101)"Soil consistence" means the degree and kind of cohesion and adhesion that a soil exhibits.
- (99)(102)"Soil series" means an official series name established by USDA-NRCS.
- (100)(103)"Soil structure" means the arrangement of primary soil particles into compound particles, peds, or clusters that are separated by natural planes of weakness from adjoining units.
- (101)(104)"Soil textural classes" means soil classification based upon size distribution of mineral particles in the fine-earth fraction less than two millimeters in diameter. The fine-earth fraction includes sand, silt, and clay particles. Sand particles are 0.05 2.0 mm in size, silt particles are 0.002 0.05 mm in size, and clay particles are less than 0.002 mm in size.
- (102) "Stream" means a body of concentrated flowing water in a natural low area or natural or manmade channel on the land surface. This includes ephemeral, intermittent, and perennial streams as those terms are defined at 15A NCAC 02B .0233(2)(d), (g), and (i), respectively, as well as streams which have been modified by channeling, culvert installation, or relocation.
- (103)(105)"Structurally sound" means a tank that has been installed in accordance with the tank manufacturer's requirements and is able to withstand a minimum uniform live loading of 150 pounds per square foot in addition to all loads to which an underground tank is normally subjected, such as dead weight of the material and soil over the tank, active soil pressure on tank walls, and the uplifting force of groundwater.
- (104)(106)"Surface water diversion" means a natural or constructed drainage feature used to divert surface water, collect runoff, and direct it to an effective outlet. Surface water diversions include waterways, berms, swales, and ditches. Surface water diversions are a type of artificial drainage.
- (105)(107)"TS-I systems" means advanced pretreatment systems approved by the Department in accordance with Section .1700 of this Subchapter that meet TS-I effluent standards in Table XXV of Rule .1201(a) of this Subchapter.
- (106)(108)"TS-II systems" means advanced pretreatment systems approved by the Department in accordance with Section .1700 of this Subchapter that meet TS-II effluent standards in Table XXV of Rule .1201(a) of this Subchapter.
- (107)(109)"Telemetry" means the ability to contact by phone, email, or another electronic medium. The telemetry unit shall continue alarm notifications to the designated party until the alarm condition is remedied or the telemetry unit is physically turned off.
- (108)(110)"Test system" means the dispersal system proposed for accepted status as part of a survey protocol identified in Rule .1706 of this Subchapter.
- (109)(111)"Third-party" means a person or entity engaged in testing or evaluation that may be compensated for their work product that is independent of the parties for whom testing or evaluation is performed and does not otherwise benefit regardless of the outcome. The third-party person or entity has knowledge of the subject area based upon relevant training and experience.
- (110)(112)"Timed dosing" means a configuration in which a specific volume of effluent is delivered to a component based upon a prescribed interval, regardless of facility water use variation over time.
- (111)(113)"Treatment media" means the media used for physical, chemical, and biological treatment in a wastewater treatment component.
- (112)(114)"Trench" means an excavation with a width less than or equal to three feet containing dispersal media and one or more laterals.

- (113)(115)"Underground utility" means any underground line, system, or infrastructure used for producing, storing, conveying, transmitting, identifying, locating, or distributing communication, electricity, gas, petroleum or petroleum products, hazardous liquids, water, steam, or sewage.
- (114)(116)"Unstable slopes" means areas showing indications of mass downslope movement such as debris flows, landslides, and rock falls.
- (115)(117)"Vertical separation" means the depth beneath the dispersal field infiltrative surface to a LC.
- (116)(118)"Warming kitchen" means a kitchen that does not meet the requirements of North Carolina Food Code, Chapters 4-1 and 4 2. contains domestic equipment and is used for plating or dispensing food prepared or cooked at another location.
- (117)(119)"Water main standards" means design criteria for pipe and pipe joints and associated installation procedures used in potable water systems and that have been approved by North Carolina DEQ Public Water Supply Section in accordance with 15A NCAC 18C.
- (118)(120)"Watertight" means that no water moves into or out of the structure or device, except through designated inlets and outlets. Watertight tanks shall demonstrate compliance with the leak testing requirements in Rule .0805 of this Subchapter.

History Note: Authority G.S. 130A-335(e) and (f); <u>S.L. 2023-77, s.2; S.L. 2024-49, s.4.11;</u> Eff. January 1, 2024.

#### **SECTION .0200 – PERMITS**

#### 15A NCAC 18E .0202 APPLICATION

(a) An application for an IP, CA, and existing system authorization IP and CA shall be submitted by the applicant to the LHD, and approved in accordance with these Rules, for each site prior to the construction, location, or relocation of a residence, place of business, or place of public assembly. LHD:

- (1) prior to the construction, location, or relocation of a residence, place of business, or place of public assembly;
- (2) prior to the construction, location, or relocation of a wastewater system; or
- (3) when an increase in DDF or wastewater strength is proposed for an existing system.

(b) An application for an existing system approval shall be submitted to the LHD prior to site modifications that require the issuance of a building permit.

(b)(c) Prior to the repair of a wastewater system, an application for a CA shall be submitted to the LHD.

 $\frac{(c)(d)}{d}$  A pending application for an IP, CA, or existing system authorization for which the LHD is awaiting action by the applicant shall expire 12 months from the date of application.

(d)(e) When an IP, CA, or existing system authorization expires or is revoked, or an application for an IP or CA expires, a new application is required.

(e)(f) For a Type V or VI system as specified in Table XXXII of Rule .1301(b) of this Subchapter, a new application shall be submitted at least 30 days prior to the OP expiring.

(f)(g) An applicant may choose to contract with an LSS to conduct a soil and site evaluation in accordance with G.S. 130A-335(a2). The soil and site evaluation shall be submitted to the LHD as part of the application process.

(g)(h) The application for an IP shall contain the following information:

- (1) name, mailing address, and phone number of the applicant and owner;
- (2) type of permit requested:
  - (A) new;
  - (B) change of use;
  - (C) expansion or increase in DDF; or
  - (D) wastewater system relocation;
- (3) site plan or plat indicating the locations of the following:
  - (A) existing and proposed facilities, structures, appurtenances, and wastewater systems;
  - (B) proposed wastewater system showing setbacks to property line(s) or other fixed reference point(s);
  - (C) existing and proposed vehicular traffic areas;
  - (D) existing and proposed water supplies, wells, springs, and water lines; and
  - (E) surface water, drainage features, and all existing and proposed artificial drainage, as applicable;
- (4) location, parcel identification number, other property identification, 911 address if known, acreage, and general directions to the property;
- (5) description of existing and proposed facilities and wastewater systems;
- (6) information needed to determine DDF and effluent strength of the facility(s) served, including number and function of individual design units, number of bedrooms and occupants per bedroom, or number of occupants;
- (7) whether wastewater other than DSE will be generated;
- (8) notification if the property includes, or is subject to, any of the following:
  - (A) previously identified jurisdictional wetlands;
  - (B) existing or proposed easements, rights-of-way, encroachments, or other areas subject to legal restrictions; or
  - (C) approval by other public agencies; and

(9) signature of applicant and owner.

(h)(i) The application for a CA shall contain:

- (1) the information required in Paragraph (g)(h) of this Rule. A site plan or plat shall not be required with the application to repair a permitted wastewater system when the repairs will be accomplished on property owned and controlled by the owner and for which property lines are identifiable in the field;
- (2) identification of the proposed use of a grinder pump or sewage pump; and
- (3) the type of the proposed wastewater system specified by the applicant.

(i)(j) The application for an existing system authorization shall contain:

- (1) name, mailing address, and phone number of the applicant and owner;
- (2) a site plan or plat indicating the locations of the existing and proposed facilities, existing wastewater systems and repair areas, existing and proposed water supplies, easements, rights-of-way, encroachments, artificial drainage, and all appurtenances;
- (3) location, parcel identification number, other property identification, 911 address if known, acreage, and directions to the property;
- (4) for reconnections, information needed to determine DDF of the facility served, including number and function of individual design units, number of bedrooms and occupants per bedroom, or number of occupants; and
- (5) signature of applicant and owner(s).

(j)(k) Submittal of a signed application shall constitute right of entry to the property by an authorized agent.

History Note: Authority G.S. 130A-335; 130A-336; 130A-337; 130A-338; <u>S.L. 2023-77, s.3; S.L. 2024-49, s.4.12;</u> Eff. January 1, 2024.

# 15A NCAC 18E .0203 IMPROVEMENT PERMIT

(a) Upon receipt of a complete application for an IP, an authorized agent shall evaluate the site to determine whether the site is suitable or unsuitable for the installation of a wastewater system in accordance with Section .0500 of this Subchapter. If the site is classified suitable, an IP shall be issued in accordance with this Subchapter. The authorized agent shall prepare dated, written documentation of the soil and site conditions required to be evaluated in Section .0500 of this Subchapter.

(b) When the site is classified suitable an authorized agent shall issue an IP for the site that includes the items contained in G.S. 130A-336(a)(1) through (6) and the following information:

- (1) DDF, number of bedrooms, maximum number of occupants or people served, and wastewater strength in accordance with Section .0400 of this Subchapter;
- (2) required effluent standard DSE, HSE, NSF/ANSI 40, TS-I, TS-II, or RCW in accordance with Table III of Rule .0402(a), Table XXV of Rule .1201(a), or Rule .1002, of this Subchapter;
- (3) all applicable setbacks and requirements in accordance with Section .0600 of this Subchapter;
- (4) description of the facility, structures, vehicular traffic areas, and other proposed improvements;
- (5) description of existing and proposed public or private water supplies, including private drinking water wells and springs and associated water lines;
- (6) a site plan or plat as defined in G.S. 130A-334 showing the existing and proposed property lines with dimensions, the location of the facility and appurtenances, the site for the proposed wastewater system and repair area, and the location of water supplies and surface water;
- (7) the proposed initial wastewater system and repair system areas and types, including <u>usable depth to a LC</u>, <del>LTARs for each system</del>, <u>LTARs</u>, and maximum trench depth that considers percent slope; and
- (8) permit conditions, such as site-specific site modifications, installation requirements, maintenance of the groundwater lowering system, etc.

(c) When the site is classified unsuitable, a signed, written report shall be provided to the applicant describing the unsuitable site characteristics and citing the applicable rule(s). If modifications or alternatives are available to support site reclassification to suitable this information shall be included in the report.

- (d) The period of validity for the permit in accordance with G.S. 130A-335(f) shall be stated on the IP.
- (e) The IP shall be transferable subject to the conditions set forth in G.S. 130A-336(a).
- (f) An IP shall be suspended or revoked if:
  - (1) the information submitted in the application is found to be incomplete, false, or incorrect;
  - (2) the site is altered and the permitted system cannot be installed or operated as permitted;
  - (3) conditions of the IP or the Rules of this Subchapter cannot be met;
  - (4) a new IP is issued for the same design unit on the same property; or
  - (5) an NOI is issued in accordance with G.S. 130A-336.1(b) or G.S. 130A-336.2(b) for the same design unit on the same property.

(g) An IP shall be applicable to both initial and repair dispersal field areas identified and approved on the <u>IP</u>. <del>IP</del> and only a CA shall be issued if wastewater system repairs are necessary.

History Note: Authority G.S. 130A-335; 130A-336; <u>S.L. 2023-77, s.4; S.L. 2024-49, s.4.13;</u> Eff. January 1, 2024.

# 15A NCAC 18E .0204 CONSTRUCTION AUTHORIZATION

(a) The applicant shall obtain a CA after an IP has been issued and prior to the construction, location, or relocation of a facility, or the construction or repair of a wastewater system.

- (b) Conditions of an IP shall be completed prior to the issuance of a CA. A CA shall be issued by an authorized agent for wastewater system installation when it is found that the IP conditions and Rules of this Subchapter are met.
- (c) A CA may be issued at the same time as the IP if no conditions on the IP are required to be completed prior to CA issuance.
- (d) Any necessary easements, rights-of-way, or encroachment agreements shall be obtained prior to the issuance of a CA.
- (e) The CA shall specify the following:
  - (1) all information required in Rule .0203(b) of this Section;
  - (2) the initial wastewater system type and layout, location of all initial wastewater system components, and design details and specifications for the following, as applicable;
    - (A) tanks;
    - (B) collection sewers; sewers, supply lines, and force mains;
    - (C) pump requirements;
    - (D) advanced pretreatment;
    - (E) distribution devices; and
    - (F) trench width, length, and depth on the downslope side of the trench;
  - (3) the nature of the Management Entity required and the minimum operation and maintenance requirements in accordance with Section .1300 of this Subchapter; and
  - (4) permit conditions, such as site-specific installation requirements, maintenance of the groundwater lowering system, etc.

(f) A CA shall be issued for each wastewater system serving a facility. Separate CAs may be issued for individual components. A building permit shall not be issued for a design unit until CAs for all components of the wastewater system serving that design unit have been issued.

(g) Prior to the issuance of a CA for a system where all or part of the system will be under common or joint control, a draft multi-party agreement between the developer and an incorporated owners' association shall be submitted to and its conditions approved by the LHD. The draft multi-party agreement shall include and address the following, as applicable:

- (1) ownership;
- (2) transfer of ownership;
- (3) maintenance;
- (4) operation;
- (5) wastewater system repairs; and
- (6) designation of fiscal responsibility for the continued satisfactory performance of the wastewater system and repair or replacement of collection, treatment, dispersal, and other components.

(h) Systems or components under common or joint control include the following:

- (1) wastewater system serving a condominium or other multiple-ownership development; or
- (2) off-site systems serving two or more facilities where any components are under common or joint ownership or control.
- (i) The CA shall be valid for a period equal to the period of validity of the IP and stated on the permit.
- (j) The CA shall be transferable subject to the conditions set forth in G.S. 130A-336(a).
- (k) A CA shall be suspended or revoked if:
  - (1) the information submitted in the application is found to be incomplete, false, or incorrect;
  - (2) the site is altered and the permitted system cannot be installed or operated as permitted;
  - (3) conditions of the CA or the Rules of this Subchapter cannot be met;
  - (4) a new CA is issued for the same design unit on the same property; or
  - (5) an NOI is issued in accordance with G.S. 130A-336.1(b) or G.S. 130A-336.2(b) for the same design unit on the same property.

History Note: Authority G.S. 130A-335; 130A-336; 130A-338; <u>S.L. 2024-49, s.4.14;</u> Eff. January 1, 2024.

#### 15A NCAC 18E .0206 EXISTING SYSTEM APPROVALS FOR RECONNECTIONS AND PROPERTY ADDITIONS

(a) <u>The LHD, an AOWE, or a certified inspector may issue</u> an approval for <u>an existing system approval reconnection when the new or</u> improved facility is within the same footprint of the previous existing facility and <u>when there is no increase in DDF or wastewater</u> strength. Approval by an authorized agent shall be issued prior any of the following: strength for the following:

- (1) a facility being reconnected to an existing system; or a reconnection for a new or improved facility; or
- (2) other site modifications as described in Paragraph (c) of this Rule. a site modification that requires a building permit, such as a swimming pool or storage shed.

(b) Approvals Existing system approvals as described in Paragraph (a) of this Rule for reconnecting a facility shall be issued by an authorized agent agent, AOWE, or certified inspector upon determination of the following:

- (1) the site complies with its OP or the wastewater system was in use prior to July 1, 1977;
- $\frac{(2)(1)}{(2)}$  there is no current or past uncorrected malfunction of the system as described in Rule .1303(a)(2) of this Subchapter;
- (3)(2) the DDF and wastewater strength for the proposed facility do not exceed that of the existing system;

(4)(3) the proposed facility or site modifications meet meets the setbacks in Section .0600 of this Subchapter; and Subchapter.

(5) the existing system is being operated and maintained as specified in G.S. 130A, Article 11, this Subchapter, and permit conditions.

(c) The issuance of the existing system approvals approval expires one year after the date of issuance. for modifications or expansions shall follow the provisions of this Rule.

(c) Prior to construction, relocation of a structure, the expansion of an existing facility's footprint, or other site modifications that require the issuance of a building permit, but that do not increase DDF or wastewater strength, an approval shall be issued by an authorized agent upon determination of the compliance of the proposed structure with setback requirements in Section .0600 of this Subchapter.
 (d) An applicant shall obtain a CA from the LHD or an NOI from an AOWE prior to starting construction for any of the following:

(1) an increase in DDF;

- (2) <u>an increase in wastewater strength; or</u>
- (3) the location of the facility is not in the same footprint as the previous existing facility.

(d)(e) For approvals issued in accordance with this Rule the authorized agent agent, AOWE, or certified inspector shall provide written documentation of the approval to the applicant. The written documentation of the approval shall include the maximum number of occupants or people served and any other information relating to the facility use, including showing the location of existing and proposed structures. describe the site modification, system use, DDF, wastewater strength, number of bedrooms, and number of occupants, and shall include a site plan showing the location, dimensions, and setbacks of existing and proposed structures to the existing system and repair area.

(e)(f) When an approval cannot be issued in accordance with this Rule, a signed, written report shall be provided by the authorized agent agent, AOWE, or certified inspector, as applicable, to the applicant describing the reasons for the denial, citing the applicable rule(s), and including notice of the right to appeal under G.S. 130A-24 and 150B. rule(s). The LHD shall include the notice of the right to appeal under G.S. 130A-24 and 150B.

(g) The owner is responsible for providing the location of the property lines and site modifications. When the existing wastewater system cannot be located, the owner is responsible for locating the existing wastewater system and providing that information to the LHD, AOWE, or certified inspector. The owner is responsible for the accuracy of the information provided on the application. The owner is responsible for ensuring that all setback requirements in Section .0600 of this Subchapter are met for the property addition.
(h) Notwithstanding this Rule, the owner of a wastewater system may elect to utilize the Affidavit for Existing Wastewater System Approval offered pursuant to G.S. 160D-1110(h1) to obtain a wastewater system approval and any necessary permits.

History Note: Authority G.S. 130A-335; 130A-337(c) and (d); <u>S.L. 2023-77, s.5; S.L. 2024-49, s.4.15;</u> Eff. January 1, 2024.

# 15A NCAC 18E .0207 ALTERNATIVE WASTEWATER SYSTEM PERMITTING OPTIONS

(a) The owner of a wastewater system may obtain an IP or a CA from the LHD or an NOI from a PE or AOWE, as applicable.

(a)(b) An applicant owner may choose to use an EOP for wastewater systems in accordance with G.S. 130A-336.1 or an AOWE in accordance with G.S. 130A-336.2. 130A-336.2 to obtain a wastewater system approval. The An EOP shall may be used if the wastewater system design requires a PE in accordance with Rule .0303(a) of this Subchapter.

(c) For an EOP, a PE may use pretreatment technologies not approved in the State provided the engineering report attached to the ATO specifies that pretreatment technology and includes the manufacturer's approval for use of the pretreatment technology for the conditions at that site.

(b)(d) Prior to the submittal of an NOI for an EOP or an AOWE system as required by G.S. 130A-336.1(b) or G.S. 130A-336.2(b), respectively, a soil and site evaluation shall be conducted in accordance these Statutes and the Rules of this Subchapter. The NOI submitted pursuant to this Rule shall include the signed and sealed soils report of the LSS or LG releasing the soils report to be used in the issuance of the NOI.

(e) The owner of an EOP system or an AOWE system shall be responsible for the following:

- (1) preventing modifications or alterations on the site, including the designated initial and repair dispersal areas;
- (2) <u>obtaining written approval from the PE or AOWE prior to making any changes to the DDF, wastewater strength, or</u> landscape position of the facility; and
- (3) identifying any drinking water wells, public water supplies, or wastewater systems on the property and on adjoining property if within the setback requirements found in Section .0600 of this Subchapter.

(c)(f) The NOI for an EOP or AOWE system shall be submitted to the LHD in the county where the facility is located by the applicant, owner, PE authorized as the legal representative of the owner, or AOWE authorized as the legal representative of the owner. The NOI shall be submitted on the common form for EOP or the common form for AOWE provided by the Department. The common forms are available by accessing the Department's website at https://ehs.ncpublichealth.com/oswp/. The forms shall include all the information specified in G.S. 130A-336.1(b) or 130A-336.2(b) and the following:

- (1) the LSS's, and LG's name, license number, address, e-mail address, and telephone number, as applicable. The installer's name, license number, address, e-mail address, and telephone number shall be provided on the EOP common form;
- (2) information required in Rule .0202 of this Section for IP and CA applications;
- (3) identification and location on the site plan of existing or proposed potable water supplies, geothermal heating and cooling wells, and groundwater monitoring wells for the proposed site. The PE or AOWE shall reference any existing permit issued for a private drinking water well, public water system as defined in G.S. 130A-313(10), or a wastewater

system on both the subject and adjoining properties to provide documentation of compliance with setback requirements in Section .0600 of this Subchapter; and

(4) proof of insurance for the PE, LSS, and LG, as applicable. Proof of insurance for the installer shall be provided with the NOI.

(d)(g) The PE or AOWE design shall incorporate findings and recommendations on soil and site conditions, limitations, site modifications, and geologic and hydrogeologic conditions specified by the LSS or LG, as applicable, and in accordance with G.S. 130A-336.1(b)(8) or G.S. 130A-336.2(b)(9), respectively. For an EOP, when the PE chooses to employ pretreatment technologies not approved in this State, the engineering report shall specify the proposed technology and the associated siting, installation, operation, maintenance, and monitoring requirements, including written manufacturer's endorsement of the proposed use.

(e)(h) The PE or AOWE shall allow for the use of Accepted Systems in accordance with G.S. 130A-336.1(e)(5) or G.S. 130A-336.2(d)(5), respectively.

(f)(i) No building permit for construction, location, or relocation shall be issued until after a decision of completeness of the NOI is made by the LHD. If the LHD fails to act within 15 business days for an EOP or within five business days for an AOWE, the common form is deemed complete.

 $(\underline{g})(\underline{j})$  If there are any changes in the site plan that can impact the wastewater system, such as moving the house or driveway, site alterations, or if the applicant chooses to change the DDF or the wastewater strength prior to wastewater system construction, a new NOI shall be submitted to the LHD. The applicant shall request in writing that the PE or AOWE invalidate the prior NOI with a signed and sealed letter sent to the applicant and LHD.

(h)(k) Construction of the wastewater system shall not commence until the system design plans and specifications have been provided to the installer and the signed and dated statement by the installer is provided to the applicant as required by G.S. 130A-.336.1(e)(4)(b) or G.S. 130A-336.2(e)(3). The applicant shall be responsible for preventing modifications or alterations of the site for the wastewater system and the system repair area before, during, and after any construction activities for the facility, unless approved by the licensed professionals.

(i)(1) Prior to the LHD providing written confirmation on the common form for the ATO completeness, the applicant, owner, PE, or AOWE shall submit the following to the LHD:

- (1) documentation that all reporting requirements identified in G.S. 130A-336.1(l) or 130A-336.2(l) have been met;
- (2) information set forth in Rule .0301(d) of this Subchapter;
- (3) system start-up documentation, including applicable baseline operating parameters for all components;
- (4) documentation by the applicant that all necessary legal agreements, including easements, encroachments, multi-party agreements, and other documents have been prepared, executed, and recorded in accordance with Rule .0301(b) and (c) of this Subchapter;
- (5) installer's name, license number, address, e-mail address, telephone number, and proof of insurance for AOWE only; and
- (6) record drawings.

 $\frac{(j)(m)}{m}$  The owner of a wastewater system approved in accordance with this Rule shall be responsible for maintaining the wastewater system in accordance with the written operation and management program required in G.S. 130A-336.1(i)(1) or 130A-336.2(i)(1) and Section .1300 of this Subchapter.

 $\frac{k}{n}$  For repair of a malfunctioning EOP or AOWE system, an NOI shall be submitted in accordance with this Rule. Rule .1306 of this Subchapter shall be followed for repair of a malfunctioning system. The Management Entity shall notify the LHD within 48 hours of the system malfunction.

(1)(0) The applicant of an EOP or AOWE system who proposes to change the use of the facility shall contact the licensed professionals on the NOI to determine whether the current system would continue to comply with the Rules of this Subchapter for the proposed change of use. The licensed professionals shall determine what, if any, modifications shall be necessary for the wastewater system to continue to comply with the Rules of this Subchapter following the proposed change of use. An NOI reflecting the change of use and any required modifications to the system shall be submitted to the LHD. The permitting process set forth in this Rule shall be followed.

(m)(p) For EOP and AOWE systems, the LHD shall:

- (1) file all EOP and AOWE documentation consistent with current permit filing procedures at the LHD;
- (2) revoke an IP or CA for a wastewater system prior to issuing written confirmation of the NOI for the same design unit on the same property, if applicable;
- (3) revoke an OP for a wastewater system prior to issuing written confirmation of an ATO for the same design unit on the same property, if applicable;
- (4) submit a copy to the Department of the common form indicating written confirmation of NOI and ATO completeness;
- (5) participate in a post-construction conference in accordance with G.S. 130A-336.1(j) or G.S. 130A-336.2(j);
- (6) review the performance and operation reports submitted and perform on-site compliance inspections of the wastewater system in accordance with Rule .1305(c) and Table XXXII of Rule .1301(b) of this Subchapter;
- (7) investigate complaints regarding EOP and AOWE systems;
- (8) issue a NOV for systems determined to be malfunctioning in accordance with Rule .1303(a)(2) of this Subchapter. The LHD shall direct the owner to contact the PE, LSS, LG, and installer, as applicable, for determination of the reason of the malfunction and development of an NOI for repairs; and
- (9) require an owner receiving a NOV to pump and haul sewage in accordance with Rule .1306 of this Subchapter.

(n)(q) The applicant may contract with different licensed professionals than those originally identified on the initial NOI to complete an EOP or AOWE project. When the applicant contracts with different licensed professionals, a revised NOI reflecting the new licensed professionals and proof of insurance shall be submitted to the LHD.

(o)(r) The applicant and all licensed professionals shall comply with all applicable federal, State, and local laws, rules, and ordinances. (s) A wastewater system that has been repaired is not required to have an additional repair area.

(t) A wastewater system permitted in accordance with G.S. 130A-336.2 shall follow the rules established by the North Carolina On-Site Wastewater Contractors and Inspectors Certification Board.

*History Note: Authority G.S. 130A-335; 130A-336.1; 130A-336.2; S.L. 2019-151, s.14; <u>S.L. 2023-77, s.6; S.L. 2024-49, s.4.16;</u> <i>Eff. January 1, 2024.* 

# SECTION .0300 - RESPONSIBILITIES

#### 15A NCAC 18E .0301 OWNERS

(a) The owner of a wastewater system shall:

- (1) comply with G.S. 130A, Article 11, the Rules of this Subchapter, and permit conditions regarding wastewater system location, including repair area;
- (2) identify property lines and fixed reference points in the field prior to the LHD site evaluation;
- (3) make the site accessible for the site evaluation described in Rule .0501 of this Subchapter;
- (4) field stake or otherwise mark the proposed facility location and all associated appurtenances, such as vehicular traffic areas, garage, swimming pool, shed, entryways, decks, etc.;
- (5) provide for pits with excavated steps or a ramp in the pit that allow for ingress and egress when necessary for a soil and site evaluation at the site as determined by the LHD or the Department in accordance with Rule .0501 of this Subchapter;
- (6) provide for system operation, maintenance, monitoring, and reporting, including access for system maintenance;
- (7) maintain artificial drainage systems, as applicable;
- (8) prevent encroachment on the initial wastewater system and repair area by utilities, structures, vehicular traffic areas, etc.;
- (9) provide documentation supporting an exemption from the minimum setback requirements in Rule .0601(a) of this Subchapter to the LHD, as applicable;
- (10) establish and maintain site-specific vegetation over the dispersal field and repair area; and
- (11) repair a malfunctioning system as necessary in accordance with this Subchapter.

(b) The entire initial wastewater system and repair area shall be on property owned or controlled by the wastewater system owner. The entire initial wastewater system and repair area shall be on property controlled by the wastewater system owner. Property is considered controlled by the wastewater system owner when the owner has an easement or encroachment agreement for the property where the wastewater system or repair area is located. An easement or encroachment agreement shall be required for the permitting of any of the following installations:

- (1) any part of the wastewater system is located in a common area with other wastewater systems;
- (2) any part of the wastewater system is located in an area with multiple or third-party ownership or control;
- (3) any part of the wastewater system is proposed to be in an off-site area; or
- (4) any part of the wastewater system and the facility are located on different lots or tracts of land and cross a property line or right-of-way.

(c) Any necessary easements, rights-of-way, or encroachment agreements shall be obtained prior to the issuance of a CA. The easement, right-of-way, or encroachment agreement shall meet the following conditions:

- (1) be appurtenant to specifically described property and run with the land;
- (2) not be affected by change of ownership or control;
- (3) remain valid for as long as the wastewater system is required for the facility that it is designed to serve;
- (4) include a description of the uses being granted and shall include ingress, egress, and regress, system installation, operation, maintenance, monitoring, and repairs and any other activity required to remain in compliance with this Subchapter, including that the easement, right-of-way, or encroachment remain free of structures, landscaping, or any other activities that would interfere with the use of the easement or encroachment for its intended purpose;
- (5) specify in a deed by metes and bounds description the area or site required for the wastewater system and repair area, including collection sewers, <u>force mains</u>, <u>supply lines</u>, tanks, raw sewage lift stations, distribution devices, and dispersal fields; and
- (6) be recorded with the register of deeds in the county where the system and facility are located.

(d) Prior to OP issuance for a system required to be designed by an authorized designer or PE, the owner shall submit to the LHD a statement signed by the authorized designer or PE specifying that the system has been installed in accordance with the permitted design. For systems designed by a PE, the statement shall be affixed with the PE seal.

History Note: Authority G.S. 130A-335; <u>S.L. 2023-77, s.7; S.L. 2024-49, s.4.17;</u> Eff. January 1, 2024.

# 15A NCAC 18E .0302 LOCAL HEALTH DEPARTMENT AND DEPARTMENT

(a) The permitting of a wastewater system shall be the responsibility of agents authorized by the Department in accordance with G.S. 130A, Article 4 and 15A NCAC 01O .0100, and registered with the North Carolina State Board of Environmental Health Specialist Examiners, as required in G.S. 90A, Article 4, unless the permit is issued in accordance with G.S. 130A-336.1 or G.S. 130A-336.2 and Rule .0207 of this Subchapter.

(b) When the wastewater system crosses county lines or the facility is in one county and the wastewater system is in another county, the LHD in the county that assesses property taxes on the facility shall implement the requirements of this Subchapter.

(c) <u>When a LHD issues a NOV to an owner of a wastewater system pursuant to this Rule, the LHD may pursue legal remedies no sooner</u> than 30 days after the date of the NOV, unless the NOV specifies a shorter time frame. The LHD shall issue an NOV to the owner in the following situations:

- (1) the wastewater system is malfunctioning in accordance with Rule .1303(a)(2) of this Subchapter;
- (2) the wastewater system creates or has created a public health hazard or nuisance by effluent surfacing, or effluent discharging into groundwater or surface waters;
- (3) the wastewater system is partially or totally destroyed, such as components that are crushed, broken, damaged, or otherwise rendered unusable or ineffective so that the component will not function as designed;
- (4) the owner does not meet the ownership and control requirements of Rule .0301(b) of this Section;
- (5) the wastewater system was installed without a permit issued in accordance with Section .0200 of this Subchapter; or
- (6) when an individual advanced pretreatment system at a single site is out of compliance in accordance with Rule .1302(f) of this Subchapter; or
- (6)(7) the facility was expanded without a permit issued in accordance with Section .0200 of this Subchapter.

(d) The authorized agent shall issue a written notice of non-compliance to the owner when the wastewater system is non-compliant with G.S. 130A, Article 11, the Rules of this Subchapter, or the performance standards or conditions in the OP or ATO.

(e) The LHD shall submit a monthly activity report to the Department every month on a form provided by the Department. The monthly activity report collects information on the number and types of permits issued by the LHD.

(e)(f) The Department shall review and approve the wastewater system, including design, layout, plans, and specifications for all wastewater systems that serve a facility with a cumulative DDF greater than 3,000 gpd, as determined in Section .0400 of this Subchapter. The Department shall also review and approve plans and specifications for the following:

- (1) IPWW systems required by this Section to be designed by a PE unless the wastewater has been determined to not be IPWW in accordance with Rule .0303(a)(17) of this Section;
- (2) advanced pretreatment or drip dispersal systems not previously approved by the Department; and
- (3) any other system so specified by the authorized agent.

(f)(g) Department review shall not be required when the cumulative DDF for the facility is greater than 3,000 gpd as determined in Section .0400 of this Subchapter and:

- (1) the wastewater system is made up of an individual wastewater system that serves an individual dwelling unit or several individual wastewater systems, each serving an individual dwelling unit; or
- (2) the wastewater system meets the following criteria:
  - (A) the individual wastewater system(s) serves individual design units with a DDF less than or equal to 1,500 gpd;
  - (B) the initial and repair dispersal fields for each individual wastewater system(s) is, at a minimum, 20 feet from any other individual wastewater system;
  - (C) the total DDF for all dispersal fields is less than or equal to 1,500 gpd per acre based on the portion of the land containing the dispersal fields; and
  - (D) the wastewater is not HSE as identified in Section .0400 of this Subchapter.

 $(\underline{g})(\underline{h})$  Department review shall not be required when a PE calculates the proposed DDF to be less than or equal to 3,000 gpd based on engineering design utilizing low-flow fixtures and low-flow technologies in accordance with Rule .0403(e) of this Subchapter. Pursuant to S.L. 2013-413, s.34, as revised by S.L. 2014-120, s.53, neither the Department nor any LHD shall be liable for a system approved or permitted in accordance with this Paragraph.

(h)(i) For systems that require Department review and approval, an IP shall not be issued by the LHD until the site plan or plat and system layout, including details for any proposed site modifications, are approved by the Department. A CA shall not be issued by the LHD until plans and specifications, submitted in accordance with Rule .0304 of this Section, are approved by the Department in accordance with these Rules and engineering practices.

(i)(j) The Department shall provide technical assistance to the LHD as needed for interpretation of this Subchapter, in accordance with the recognized principles and practices of soil science, geology, engineering, and public health.

(k) The LHD shall adhere to G.S. 130A, Article 11, the Rules of this Subchapter, and all written guidance and interpretations from the Department.

History Note: Authority G.S. 130A-335; <u>S.L. 2024-49, s.4.18;</u> Eff. January 1, 2024.

# 15A NCAC 18E .0303 LICENSED OR CERTIFIED PROFESSIONALS

(a) Any wastewater system that meets one or more of the following conditions shall be designed by a PE if required in G.S. 89C:

- (1) the system has a DDF greater than 3,000 gpd, as determined in Section .0400 of this Subchapter, except where the system is limited to an individual wastewater system serving an individual dwelling unit or multiple individual wastewater systems, each serving an individual dwelling unit;
- (2) the system requires advanced pretreatment or drip dispersal and is not a system approved under Sections .1500, .1600, or .1700 of this Subchapter;
- (3) pressure dispersal systems that require pumping more than 500 feet horizontally or more than 50 feet of net elevation head;
- (4) pressure dosed gravity distribution systems that require pumping more than 1,000 feet horizontally or more than 100 feet of net elevation head;
- (5) dosing systems or force mains that have one or more intermediate high points greater than five feet;
- (6) the system requires pumping downhill to a pressure dosed gravity or pressure dispersal field where the volume of the supply line that could drain to the dispersal field between doses exceeds 25 percent of the required dose volume;
- (7) pressure dispersal systems and pressure dosed gravity systems with a DDF greater than 600 gpd serving a single design unit;
- (8)(7) pressure dispersal systems where there is more than 15 percent variation in line length. The 15 percent variation shall be measured by comparing the longest line length to the shortest line length in any dispersal field;
- (9) two or more septic tanks or advanced pretreatment units, each serving a separate design unit, and served by a common dosing tank;
- (10)(8) a STEP system with a pressure sewer or other pressure sewer system receiving effluent from two or more pump tanks;
- (11) an adjusted DDF is proposed based on the use of low flow fixtures or low flow technologies in accordance with Rule .0403(e) of this Subchapter;
- (12)(9) the system requires use of sewage pumps prior to the septic tank or other pretreatment system, except for systems governed by the North Carolina Plumbing Code or which consist of grinder pumps and associated pump basins that are approved and listed in accordance with standards adopted by NSF International;
- (13)(10) an individual system is required to use more than one pump or siphon in a single pump tank. Examples include dual pumps as set forth in Rule .1101(b) of this Subchapter;
- (14)(11) the system includes a collection sewer prior to the septic tank or other pretreatment system serving two or more design units, except for systems governed by the North Carolina Plumbing Code;
- (15)(12) the wastewater system includes structures that have not been pre-engineered;
- (16)(13) the proposed pump model is not listed by a third-party electrical testing and listing agency;
- (17)(14) the system is designed for the collection, treatment, and dispersal of IPWW, except under the following circumstances:
  - (A) the Department has determined that the wastewater generated by the proposed facility has a pollutant strength that is lower than or equal to DSE and does not require specialized treatment or management. This determination shall be made based on a review of the wastewater generating process, wastewater characteristic data, and material safety data sheets, as compared to DSE; or
  - (B) the Department has approved a treatment system or process and management method proposed by the facility owner that generates effluent with a pollutant strength which is lower than or equal to DSE. This approval shall be based on a review of documentation provided in conjunction with prior project specific reviews or a PIA approval. This approval shall be based on data from other facilities, management practices, and other information provided by the owner;
- (18)(15) the wastewater system is designed for RCW;
- (19)(16) any wastewater system designed by a licensed professional that has been determined to be within the practice of engineering in accordance with G.S. 89C-3(6) by the North Carolina Board of Examiners for Engineers and Surveyors;
- (20)(17) any wastewater system approved in accordance with Sections .1500, .1600, and .1700 of this Subchapter that requires in the RWTS or PIA Approval that the system be designed by a PE; and
- (21)(18) any system or system component where the Rules of this Subchapter provide for an engineer to propose alternative materials, capacity determination, or performance requirements; and requirements.
- (22) any other system so specified by the LHD, based on wastewater system complexity and LHD's experience with the proposed system type.

(b) A PE, in accordance with G.S. 89C, may propose an alternative design for a facility projected to generate HSE in accordance with Rule .0401(h) of this Subchapter. The alternative design shall include supporting documentation showing that the proposed system design will meet DSE in Table III of Rule .0402(a) of this Subchapter. The alternative design shall be reviewed and approved by the Department unless the system has been approved in accordance with Section .1700 of this Subchapter.

(c) Prior to the issuance of an IP or CA, the LSS or LG shall sign and affix their seal to the consent required by this Rule, which shall be attached to the plans and specifications submitted to the LHD.

(e)(d) Plans and specifications for the use of a groundwater lowering system to comply with the vertical separation to a SWC shall be prepared by a licensed professional if required in G.S. 89C, 89E, or 89F. Prior to the issuance of an IP or CA, the plans and specifications shall be reviewed and approved by the authorized agent if the plans and specifications meet the requirements of Rules .0504 and .0910 of this Subchapter and accepted design practices.

(d)(e) An installer shall construct, install, or repair wastewater systems as required by G.S. 90A, Article 5. The installer shall be responsible for the following:

(1) certification at the required level according to the system design specifications as required by G.S. 90A, Article 5;

- (2) notification to the LHD upon completion of the system installation and each stage requiring inspection as conditioned on a CA;
- (3) participation in a preconstruction conference when specified in the CA or by the RWTS or PIA Approval;
- (4) participation during the inspection of the wastewater system by the authorized agent;
- (5) participation during the post-construction conference and all other requirements when the wastewater system is permitted in accordance with Rule .0207 of this Subchapter and G.S. 130A-336.1 or G.S. 130A-336.2; and
- (6) final cover of the system after LHD approval. The wastewater system shall be in the same condition when covered as when approved.

(e)(f) The Management Entity, or its employees, shall hold a valid and current certificate or certifications as required for the system from the Water Pollution Control Systems Operators Certification Commission. Nothing in this Subchapter shall preclude any requirements for system Management Entities in accordance with G.S. 90A, Article 3.

(f)(g) Nothing in this Rule shall be construed as allowing any licensed professional to provide services for which he or she has neither the educational background, expertise, or license to perform, or is beyond his or her scope of work and the applicable statutes for their respective professions.

 $(\underline{g})(\underline{h})$  The PE, AOWE, or authorized designer shall provide a written statement to the owner specifying that construction is complete and in accordance with approved plans, specifications, and modifications. This statement shall be based on periodic observations of construction and a final inspection for design compliance. Record drawings shall be provided to the owner and LHD when any change has been made to the wastewater system installation from the approved plans.

(i) A LHD may not require any system other than those identified in Paragraph (a) of this Rule to be designed by a PE.

History Note: Authority G.S. 89C; 89E; 89F; 90A; 130A-335; <u>S.L. 2023-77, s.8; S.L. 2024-49, s.4.19;</u> Eff. January 1, 2024.

# SECTION .0400 – DESIGN DAILY FLOW AND EFFLUENT CHARACTERISTICS

# 15A NCAC 18E .0401 DESIGN DAILY FLOW

(a) The minimum DDF for dwelling units shall be <u>120 gpd per bedroom or 60 gpd per person when occupancy exceeds two persons</u> per bedroom, whichever is greater. based on:

- (1) 175 gpd for a one bedroom dwelling unit with no more than two occupants and 400 square feet of living space or less; or
- (2) 120 gpd per bedroom with a minimum of 240 gpd per dwelling unit or 60 gpd per person when occupancy exceeds two persons per bedroom, whichever is greater.
- (b) DDF for facilities other than dwelling units shall be in accordance with Table II as follows:

Facility type	Design daily flow	
Commercial		
Airports, railroad stations, bus and ferry terminals, etc.	5 gal/traveler, food preparation not included	
Barber shops	50 gal/chair	
Bars, cocktail lounges∞	20 gal/seat, food preparation not included	
Beauty shops, style shops, hair salons	125 gal/chair	
Bed and breakfast homes and inns	Dwelling unit DDF based on Paragraph (a) of this Rule plus	
	120 gal/rented room which includes the following:	
	Meals served to overnight guests	
	Laundry for linens	
	150 gal/room with cooking facilities in individual rooms	
Event Center∞	5 gal/person with toilets and hand sinks up to 4 hrs	
	10 gal/person with toilets and hand sinks up to 8 hrs	
	15 gal/person with toilets and hand sinks greater than 8 hrs	
	Add 5 gal/person with full kitchen	
Markets open less than four days/week, such as a flea	30 gal/stall or vendor, food preparation not included	
market or farmers market		
Marinas with no holding tank discharge included	30 gal/boat slip, with bathhouse	
	10 gal/boat slip, wet slips or slips on dock	
	5 gal/boat slip, dry storage or warehouse	
Motels/hotels	120 gal/room includes the following:	
	No cooking facilities in individual rooms other than a microwave or other	
	similar devices	
	No food service or limited food service establishment	
	Laundry for linens	
	150 gal/room with cooking facilities in individual rooms	

#### **TABLE II.** Design daily flow for Facilities

Offices and factories with no IPWW included	12 gal/employee/≤ 8 hr shift
Offices and factories with no fr w w included	Add 2 gal/employee/ $\leq$ 8 in sint Add 2 gal/employee/hr for more than 8 hr shift
	Add 10 gal/employee for showers
Stores, shopping centers, and malls	100 gal/1,000 ft <sup>2</sup> of retail sales area, food preparation not included
Warehouse that are not retail sales warehouses	100 gal/loading bay or
	12 gal/employee/ $\leq 8$ hr shift
	Add 2 gal/employee/hr for more than 8 hr shift
Storage warehouse including self-storage facilities and	12 gal/employee/ $\leq$ 8 hr shift
does not include caretaker residence	Add 2 gal/employee/hr for more than 8 hr shift
Alcoholic beverage tasting areas with no process	200 gal/1,000 ft <sup>2</sup> of tasting area floor space and includes glass washing
wastewater included	equipment
	Food preparation and food clean up not included
	12 gal/employee/≤ 8 hr shift
Camps/Campgrounds	
Summer camps with overnight stays*	60 gal/person, applied as follows:
	15 gal/person/food preparation
	20 gal/person/toilet facilities
	10 gal/person/bathing facilities
	15 gal/person/laundry facilities
Day camps not inclusive of swimming area bathhouse*	20 gal/person and
	5 gal/meal served with multiuse service or
	3 gal/meal served with single-service articles
Temporary Labor Camp or Migrant Housing Camp with	60 gal/person, applied as follows:
overnight stays*	15 gal/person/food preparation
	20 gal/person/toilet facilities
	10 gal/person/bathing facilities
	15 gal/person/laundry facilities
Travel trailer or RV in an RV park*	100 gal/space
Recreational Park Trailer or Park Model Trailer 400 ft <sup>2</sup> or	<del>150</del> <u>120</u> gal/space
less in an RV park* Bathhouse for campsites and RV park sites with no water	70 gal/campsite
and sewer hook ups with a maximum of four people per	70 gal/campsite
campsite	
Food preparation facilities	
Food Establishments with multiuse articles*	25 gal/seat <del>or 25 gal/15 ft<sup>2</sup> of floor space</del> open 6 hrs/day or less
1 ood Establishillenis with multiuse differes	40 gal/seat or 40 gal/15 $ft^2$ of floor space open 6 to 16 hrs/day
	Add 4 gpd/seat for every additional hour open beyond 16 hrs
Food Establishments with single service articles*	20 gal/seat <del>or 20 gal/15 ft<sup>2</sup> of floor space</del> open 6 hrs/day or less
g	30 gal/seat or 30 gal/15 $ft^2$ of floor space open 6 to 16 hrs/day
	Add 3 gpd/seat for every additional hour open beyond 16 hrs
Food stand with up to eight seats, mobile food units, and	50 gal/100 ft <sup>2</sup> of food stand, food unit, or food prep floor space and
commissary kitchens*	12 gal/employee/ $\leq 8$ hr shift
	Add 2 gal/employee/hr for more than 8 hr shift
Other food service facilities*	5 gal/meal served with multiuse articles
	3 gal/meal served with single service articles
Meat markets or fish markets with no process wastewater	50 gal/100 ft <sup>2</sup> of floor space and
included*	12 gal/employee/≤ 8 hr shift
	Add 2 gal/employee/hr for more than 8 hr shift
Health care and other care institutions	
Hospitals*	300 gal/bed
Rest homes, assisted living homes, group homes, and	150 gal/bed with laundry
nursing homes*	75 gal/bed without laundry
	Add 60 gal/resident employee with laundry
Day care facilities Family child care home or child care	15 gal/person open $\leq$ 12 hr shift without laundry
centers	Add 1 gal/person/hr open for more than 12 hrs per day
	Add 5 gal/person with full kitchen
Group homes, drug Drug rehabilitation, mental health,	75 gal/person with laundry
and other care institutions	Add 12 gal/employee/ $\leq 8$ hr shift or 60 gal/resident employee
	Add 2 gal/employee/hr for more than 8 hr shift
Orphanages	60 gal/student or resident employee with laundry
Public access restrooms	

Convenience store, service station, truck stop*	250 gal/toilet or urinal meeting the following:
	Open less than 16 hrs/day
	Food preparation not included
	Retail space not included
	325 gal/toilet or urinal meeting the following:
	Open 16 to 24 hrs/day
	Food preparation not included
	Retail space not included
Highway rest areas and visitor centers*	325 gal/toilet or urinal or
	10 gal/parking space, whichever is greater
Recreational facilities	
Bowling center	50 gal/lane, food preparation not included
Community center, gym⊕	5 gal/person plus 12 gal/employee/≤ 8 hr shift
	Add 2 gal/employee/hr for more than 8 hr shift or
	50 gal/100 ft <sup>2</sup> , whichever is greater
Country club or golf course	10 gal/person
, ,	12 gal/employee/ $\leq 8$ hr shift
	Add 2 gal/employee/hr for more than 8 hr shift
	3 gal/person for convenience stations
	Food preparation not included
Fairground	250 gal/toilet or urinal
Fitness center, spas, karate, dance, exercise	50 gal/100 ft <sup>2</sup> of floor space used by clientele
Thirds center, spus, karate, dance, excluses	5 gal/person
	Add 10 gal/person for showers
	Food preparation not included
Recreational park, State park, county park, and other	10 gal/parking space
similar facilities with no sports facilities	10 gal/parking space
Outdoor sports facilities, mini golf, batting cages, driving	250 gal/toilet or urinal, 5 gal/seat, or 10 gal/parking space, whichever is
ranges, motocross, athletic park, ball fields, stadium, and	greater
other similar facilities	Food preparation not included
Auditorium, theater, amphitheater, drive-in theater	2 gal/seat or 10 gal/parking space, whichever is greater
-	Food preparation not included
Swimming pools and bathhouses	5 gal/person domestic waste only, bathing load of pool may be used as
	an alternative method of sizing
Sports facilities courts or other similar facilities	250 gal/toilet or urinal or 50 gal/court, whichever is greater
Institutions	
Church or other religious institution*	2 gal/seat sanctuary only
charen er euler rengieus montation	3 gal/seat with warming kitchen in same structure as sanctuary
	5 gal/seat with full kitchen in same structure as sanctuary
	5 gar sout with full knoholi in same structure as schottary
Public or private accomply halls used for respective	2 gal/person with toilets and hand sinks
Public or private assembly halls used for recreation,	
regularly scheduled meetings, events, or amusement∞ *	
For aburabas, flow shall be in addition to construct	3 gal/person with addition of a warming kitchen
For churches, flow shall be in addition to sanctuary	
structure flow	3 gal/person with addition of a warming kitchen
structure flow Schools	3 gal/person with addition of a warming kitchen 5 gal/person with full kitchen
structure flow	3 gal/person with addition of a warming kitchen 5 gal/person with full kitchen 6 gal/student with no cafeteria or gymnasium
structure flow Schools	3 gal/person with addition of a warming kitchen 5 gal/person with full kitchen 6 gal/student with no cafeteria or gymnasium 9 gal/student with cafeteria only
structure flow Schools	3 gal/person with addition of a warming kitchen 5 gal/person with full kitchen 6 gal/student with no cafeteria or gymnasium 9 gal/student with cafeteria only 9 gal/student with a gymnasium only
structure flow Schools Day schools*	3 gal/person with addition of a warming kitchen 5 gal/person with full kitchen 6 gal/student with no cafeteria or gymnasium 9 gal/student with cafeteria only 9 gal/student with cafeteria only 12 gal/student with cafeteria and gymnasium
structure flow Schools	3 gal/person with addition of a warming kitchen 5 gal/person with full kitchen 6 gal/student with no cafeteria or gymnasium 9 gal/student with cafeteria only 9 gal/student with a gymnasium only

\* Facility has potential to generate HSE.

∞Designer shall use the maximum building occupancy assigned by the local fire marshal in calculating DDF unless another method for determining DDF is proposed, including the justification for not using the maximum building occupancy.

(c) The minimum DDF from any facility other than a dwelling unit shall be 100 gpd. For facilities with multiple design units, the minimum DDF shall be 100 gpd per design unit. The DDF of the facility shall be the sum of all design unit flows.

(d) DDF determination for wastewater systems with facilities not identified in this Rule shall be determined using available water use data, capacity of water-using fixtures, occupancy or operation patterns, and other measured data from the facility itself or a comparable facility.

(e) Where laundry is not specified for a facility in Table II, but is proposed to be provided, the DDF shall be adjusted to account for the proposed usage and machine water capacity. The applicant or a licensed professional shall provide cut-sheets for laundry machines proposed for use in facilities.

(f) HVAC unit or ice machine condensate, gutter or sump pump discharge, water treatment system back flush lines, or similar incidental flows shall not discharge to the wastewater system, unless a PE designs the wastewater system for these flows.

(g) Unless otherwise noted in Table II, the DDF per unit includes employees.

(h) Food service facilities and other facilities that are projected to generate wastewater with constituent levels greater than DSE, as defined in Rule .0402 of this Section, are identified in Table II with a single asterisk (\*) as HSE. Any facility that has a food service component that contributes 50 percent or more of the DDF shall be considered to generate HSE. Determination of wastewater strength shall be based on projected or measured levels of one or more of the following: BOD, TSS, FOG, or TN. Table III of Rule .0402(a) of this Section identifies the constituent limits for DSE.

(i) Wastewater with constituents other than those listed in Table III of Rule .0402(a) of this Section may be classified as IPWW as defined in G.S. 130A-334(2a) on a site-specific basis.

(j) A request for an adjusted DDF shall be made in accordance with Rule .0403 of this Section.

History Note: Authority G.S. 130A-335(e); S.L. 2013-413, s.34; S.L. 2014-120, <del>s.53;</del> <u>S.L. 2023-77, s.9; S.L. 2024-49, s.4.20;</u> Eff. January 1, 2024.

#### 15A NCAC 18E .0402 SEPTIC TANK EFFLUENT CHARACTERISTICS

(a) Septic tank effluent standards for DSE shall be as set forth in Table III of this Paragraph. Effluent that exceeds these standards for any constituent shall be considered HSE. When measured, effluent characteristics shall be based on at least two effluent samples collected during normal or above-normal operating periods. A normal period is when the occupancy, operation, or use of the facility is average when compared to the occupancy, operation, or use over a time frame of a minimum of one year. The samples shall be taken from the existing or a comparable facility on non-consecutive days of operation. A comparable facility is based on documentation showing that the hours of operation, floor plan, water use practices, water-using fixtures, location, etc., are similar to the facility listed in the application. The samples shall be analyzed for a minimum of BOD<sub>5</sub>, TSS, TN, and FOG.

Constituent	Maximum DSE mg/L
BOD	≤ <b>3</b> 50
TSS	$\leq 100$
TN* <u>TKN</u>	≤ 100
FOG	$\leq$ 30

Table III. Septic tank effluent standards for DSE

\*TN is the sum of TKN, nitrate nitrogen, and nitrite nitrogen

(b) Designs for facilities that generate HSE or when an adjusted DDF is proposed in accordance with Rule .0403 shall address the issue of wastewater strength in accordance with one of the following:

- (1) Wastewater systems that meet one of the following criteria shall utilize advanced pretreatment, designed in accordance with Rule .1201(b) of this Subchapter, to produce DSE or better prior to dispersal:
  - (A) DDF greater than 1,500 gpd and HSE;
  - (B) any proposed flow reduction in accordance with Rule .0403 of this Section where the DDF is greater than 1,500 gpd; or
  - (C) any proposed flow reduction in accordance with Rule .0403 of this Section with projected or measured effluent characteristics that exceed DSE as set forth in Table III of this Rule; or
- (2) A licensed professional, in accordance with G.S. 89C, 89E, or 89F, may justify not using advanced pretreatment by providing the following, as applicable:
  - (A) the system design is determined based upon a mass loading adjusted LTAR calculated using site-specific LTAR and projected or measured BOD<sub>5</sub> and TSS values. The adjusted LTAR calculations shall be done as follows:

	MLAF =	$300/(BOD_5 + TSS)$ or one, whichever is smaller
	ALTAR =	MLAF x LTAR
Where	MLAF =	mass loading LTAR adjustment factor
	BOD <sub>5</sub> =	measured or projected
	TSS =	measured or projected

LTAR=	LTAR assigned by the authorized agent for DSE in accordance with this
	Subchapter
ALTAR =	adjusted LTAR

- (B) site-specific nitrogen migration analysis when projected or measured effluent total nitrogen levels are greater than 100 mg/L. Analysis shall demonstrate that the nitrate-nitrogen concentration at the property line will not exceed 10 mg/L; and
- (C) additional pretreatment to reduce FOG to less than or equal to 30 mg/L, including justification for the proposed pretreatment method.

(c) The requirements of Paragraph (b) shall not apply if the effluent for a specific facility identified in Rule .0401 of this Section as HSE has been measured in accordance with Paragraph (a) of this Rule and shown to be DSE.

History Note: Authority G.S. 130A-335(e); S.L. 2013-413, s.34; S.L. 2014-120, s.53; <u>S.L. 2023-77, s.10;</u> Eff. January 1, 2024.

#### SECTION .0500 - SOIL AND SITE EVALUATION

#### 15A NCAC 18E .0504 SOIL WETNESS CONDITION

(a) SWC, such as those caused by a seasonal high-water table, a perched water table, tidal water, seasonally saturated soil, or by lateral water movement, shall be determined by field observations of soil wetness indicators as follows:

- (1) the presence of colors with a value 4 or more and a chroma 2 or less using the Munsell Soil Color Book at greater than or equal to two percent of soil volume as redox depletions or as the matrix of a horizon. Colors of chroma 2 or less that are lithochromic features shall not be considered indicative of a SWC; or
- (2) the observation or indication of saturated soils, a perched water table, or lateral water movement flowing into a bore hole, monitoring well, or open excavation above a less permeable horizon, that may occur without the presence of colors with a value 4 or more or chroma 2 or less at greater than or equal to two percent of soil volume as redox depletions or as the matrix of a horizon.
- (3) The shallowest depth to SWC determined in this Paragraph shall be used.

(b) Initial site suitability as to SWC shall be determined by field observations of soil wetness indicators in accordance with Paragraph (a) of this Rule. Sites where the SWC is less than 12 inches below the naturally occurring soil surface, or less than 18 inches if more than six inches of Group I soils are present, shall be considered unsuitable with respect to SWC.

(c) Monitoring or modeling procedures as set forth in this Rule may be used to reclassify the site as suitable with respect to SWC.

(d) Monitoring or modeling procedures as set forth in this Rule shall be required when the owner proposes to use a wastewater system requiring a greater depth to a SWC than the depth observed by soil wetness indicators in accordance with Paragraph (a) of this Rule.

(e) Modeling procedures as set forth in this Rule shall be required when the owner proposes to use sites with Group III or IV soils within 36 inches of the naturally occurring soil surface with artificial drainage, or on sites when fill is proposed to be used in conjunction with an artificial drainage system.

(f) Monitoring or modeling procedures may include the following:

- (1) direct monitoring procedure as set forth in Paragraph (g) of this Rule;
- (2) modeling procedure as set forth in Paragraph (h) of this Rule;
- (3) monitoring and modeling procedure as set forth in Paragraph (i) of this Rule; or
- (4) other modeling procedures as set forth in Paragraph (j) of this Rule.

(g) The direct monitoring procedure involves determining the SWC by observation of water surface elevations in wells during periods of high-water in accordance with the following:

- (1) no later than 30 days prior to the start of the monitoring period, the owner shall notify the LHD of the intent to monitor water surface elevations by submitting a proposal prepared by a licensed professional, if required in G.S. 89C, 89E, or 89F, that includes a site plan, well and soil profile at each monitoring site, and a monitoring plan as follows:
  - (A) the site plan shall include the proposed sites for wastewater systems, the longitude and latitude of the site, the location of monitoring wells, and all drainage features that may influence the SWC. The site plan shall also specify any proposed fill and drainage modifications;
  - (B) the monitoring plan shall include the proposed number, installation depth, screening depth, soil and well profile, materials, and installation procedures for each monitoring well. A minimum of three water level monitoring wells shall be installed for water surface observation at each site. Sites handling systems with a DDF greater than 600 gpd shall have one additional well per 600 gpd increment. Well locations shall include portions of the initial and repair dispersal field areas containing the most limiting soil and site conditions. The monitoring plan shall also provide for monitoring of the water surface elevations in the wells and all precipitation at the site; and
  - (C) notification of whether the owner or a licensed professional will perform the monitoring, including the name of the licensed professional, if applicable.
- (2) prior to installation of the monitoring wells, the authorized agent shall approve the plan. Plan approval shall be based upon a site visit and compliance with this Rule. If the plan is denied, a signed, written report shall be provided to the

owner that describes the reasons for denial, the changes necessary for approval of the plan, and notice of the right to appeal under G.S. 130A-24 and 150B;

- (3) wells shall extend a minimum of five feet below the naturally occurring soil surface, or existing ground surface for existing fill determined in accordance with Rule .0909(d) of this Subchapter, except that wells that extend down only 40 inches from the ground surface may be used if a continuous record of the water table is provided for a minimum of half of the monitoring period. One or more shallower wells may be required on sites where shallow lateral water movement or a perched SWC is anticipated based on the site investigation;
- (4) the water elevation in the monitoring wells shall be recorded daily from January 1 to April 30, taken at the same time during the day, plus or minus three hours. Rain gauges shall be located within two miles of the site. Daily rainfall measurements shall also be recorded from December 1 through April 30; and
- (5) the most recent information available from the SCO shall be used to determine the recurrence frequency of the total amount of rainfall at the site for the 120-day period ending April 15 based upon the site's historic rainfall record. This shall be done when the 120-day cumulative rainfall for the monitoring period ending on April 15 equals or exceeds the site's historic rainfall for the same period with a 30 percent frequency. The recurrence frequency shall be determined with one of the following methods:
  - (A) the licensed professional shall determine the 120-day SPI for April 15 by using the Integrated Water Portal located on the SCO's website at: http://climate.ncsu.edu/water/map. The licensed professional shall click on the map pixel that corresponds closest to the site's location. The Department will assist in obtaining this information upon request; or
  - (B) the recurrence frequency of the site's cumulative precipitation for the 120-day monitoring period ending on April 15 shall be determined for the site on a case-by-case basis from the most recent master grid provided to the Department by the SCO. The master grid contains probability distribution parameters that shall be used by the Department based upon guidance from the SCO. Based on the master grid, the Department shall derive the recurrence frequency values for the grid point that corresponds closest to the site's latitude and longitude.
- (6) The SWC shall be determined by the shallowest level that is continuously saturated for the number of consecutive days during the January through April well monitoring period shown in Table VIII as follows:

<b>TABLE VIII.</b> Rainfall SPI and exceedance probability during monitoring season related to number of consecutive days of continuous
saturation

April 15 SPI 120-day range	Recurrence frequency range	Number of consecutive days of
	120-day cumulative April 15 rainfall	continuous saturation for SWC
SPI -0.543 to 0	30% to 49.9% duration	3 days or 72 hours
SPI 0 to 0.545	50% to 69.9% duration	6 days or 144 hours
SPI 0.546 to 0.864	70% to 79.9% duration	9 days or 216 hours
SPI ≥ 0.865	80% to 100% duration	14 days or 336 hours

(7) If monitoring well data is collected during monitoring periods that span multiple years, the year that yields the shallowest SWC shall apply.

(h) The modeling procedure may be used to determine SWC by using DRAINMOD, a groundwater simulation model, to predict daily water levels over a minimum 30-year period using site-specific input parameters as outlined in the DRAINMOD User's Guide. The SWC shall be determined as the shallowest level predicted by DRAINMOD to be saturated for a 14-day continuous period between January 1 and April 30 with a recurrence frequency of 30 percent, an average of a minimum of nine years in 30, and in accordance with the following:

- (1) weather input files shall consist of hourly rainfall and daily temperature data collected over the entire period of record but for a minimum of a 30-year period from a measuring station site, such as the National Weather Service or SCO. The measuring station used shall be the station located closest to the owner's site;
- (2) soil and site inputs for DRAINMOD shall include the following:
  - (A) soil input file with the soil moisture characteristic curve and data for the soil profile that is closest to the described soil profile that is present on the site;
  - (B) soil horizon depths determined on site;
  - (C) site measured or proposed drain depth and spacing, and drain outlet elevation;
  - (D) in-situ Ksat measurements for a minimum of three representative locations on the site and at each location for the three most representative soil horizons within five feet of the surface. In-situ Ksat measurements shall be for one representative soil horizon at or above redoximorphic depletion features and two representative soil horizons at and below redoximorphic concentration features at each location on the site;
  - (E) all other model parameters based upon the DRAINMOD User's Guide; and
  - (F) a sensitivity analysis shall be conducted for the following model parameters: soil input files for a minimum of two other most closely related soil profiles; in-situ Ksat of each horizon; drain depth and spacing; and surface storage and depth of surface flow inputs.

The sensitivity analysis shall be used to evaluate the range of soil and site characteristics for choosing input parameters related to the soil profiles, Ksat input values based upon the range of in-situ Ksat values measured on the site, and

inputs for surface and subsurface drainage features based upon the range of possible elevations and distances that occur or may occur after installation of improvements. The sensitivity analysis shall establish which parameters are most critical for determination of the depth to SWC. Conservative values for the most critical parameters shall be used in applying the model to the site;

- (3) for sites designed to receive over 600 gpd, the SWC determination using DRAINMOD shall take into consideration the impact of wastewater application on the projected water table surface; and
- (4) the groundwater simulation analysis shall be prepared and submitted to the LHD by licensed professionals, if required in G.S. 89C, 89E, or 89F, qualified to use DRAINMOD by training and experience. The LHD shall submit the groundwater simulation analysis to the Department for technical review prior to approval of the SWC determination.

(i) The monitoring and modeling procedure is a combination of the direct monitoring procedure and the modeling procedure. The SWC shall be determined as the shallowest level predicted by DRAINMOD to be saturated for a 14-day continuous period between January 1 and April 30 with a recurrence frequency of 30 percent, an average of a minimum of nine years in 30, and in accordance with the following:

- (1) the procedures set forth in Paragraph (g) shall be used to monitor water surface elevation and precipitation. The rain gauges and monitoring wells required by Subparagraph (g)(4) shall use a recording device and a data file that is DRAINMOD compatible. The recording devices shall record rainfall hourly or daily and well water levels daily. The data file shall be submitted with the report to the LHD;
- (2) DRAINMOD shall be used to predict daily water levels. The DRAINMOD modeling shall be in accordance with the following:
  - (A) weather input files shall be developed from daily temperature and hourly or daily rainfall data collected over a minimum 30-year period from a measuring station, such as the National Weather Service or SCO. The measuring station used shall be the station located closest to the site. Daily maximum and minimum temperature data for the December 1 through April 30 monitoring period shall be obtained from the closest available weather station;
  - (B) soil and site inputs for DRAINMOD, including a soils data file closest to the soil series identified, depths of soil horizons, in-situ Ksat of each horizon, depth and spacing of drainage features, and depression storage shall be selected in accordance with procedures outlined in the DRAINMOD User's Guide;
  - (C) inputs shall be based upon site-specific soil profile descriptions. Soil and site input factors shall be adjusted during the model calibration process to achieve the best possible fit as indicated by the least squares analysis of the daily observations over the whole monitoring period and to achieve the best possible match between the shallowest water table depth during the monitoring period that is saturated for 14 consecutive days, measured vs. predicted. The mean absolute deviation between measured and predicted values shall be no greater than six inches during the monitoring period;
  - (D) for sites intended to receive greater than 1,500 gpd, the SWC determination using DRAINMOD shall take into consideration the impact of wastewater application on the projected water table surface; and
  - (E) the DRAINMOD analysis shall be prepared and submitted to the LHD by licensed professionals, if required in G.S. 89C, 89E, or 89F, qualified to use DRAINMOD by training and experience. The LHD or owner may request a technical review by the Department prior to approval of the SWC determination.

The monitoring and modeling procedure may also be used to re-evaluate a SWC that was previously evaluated by the direct monitoring procedure.

(j) Modeling procedures other than those set forth in this Rule may be used to determine SWC upon approval by the Department. Other modeling procedures shall be approved if the following requirements are met:

- (1) the modeling procedures use daily water levels or weather records over a 30-year period to predict future daily water levels;
- (2) the proposed model and prediction are shown to be as accurate as the prediction from DRAINMOD, calculated in accordance with Paragraph (h) of this Rule; and
- (3) documentation is provided in accordance with Rule .0509(c) of this Section.

(k) A report of the investigations made for the direct monitoring procedure, modeling procedure, or monitoring and modeling procedure in accordance with Paragraphs (g), (h), or (i) of this Rule shall be prepared prior to approval of the SWC determination. A request for technical review of the report by the Department shall include digital copies of monitoring data, model inputs, output data, and graphic results, as applicable.

History Note: Authority G.S. 130A-335(e); <u>S.L. 2023-77, s.11;</u> Eff. January 1, 2024.

# 15A NCAC 18E .0506 SAPROLITE

(a) Sites classified unsuitable due to depth to saprolite or other LC may be reclassified suitable in accordance with this Rule.

(b) <u>Saprolite shall be evaluated for suitability by the LHD</u> Sites with saprolite shall be classified as suitable if an investigation of the site using pits at locations approved by the authorized agent agent. An LSS, other than an LSS employed as an authorized agent, may use either borings or pits to evaluate saprolite. Sites with saprolite shall be classified as suitable if confirms that the following conditions are met:

- (1)a 24-inch minimum vertical separation shall be maintained in saprolite from the infiltrative surface to an unsuitable LC, unless any of the vertical separation consists of a suitable soil horizon, in which case, the 24-inch separation may be calculated based on one inch of suitable soil being equivalent to two inches of saprolite; and
- the following physical properties and characteristics shall be present in the saprolite below the proposed infiltrative (2)surface:
  - (A) the saprolite texture as determined in the field by hand texturing samples of each horizon shall be sand, loamy sand, sandy loam, loam, or silt loam;
  - the clay mineralogy shall be suitable in accordance with Rule .0503(3) of this Section; (B)
  - (C) greater than two-thirds of the saprolite by volume shall have a moist consistence of loose, very friable, friable, or firm:
  - (D) the saprolite wet consistence shall be nonsticky or slightly sticky and nonplastic or slightly plastic;
  - the saprolite shall be in an undisturbed, naturally occurring state; (E)
  - (F) the saprolite shall have no open and continuous joints, quartz veins, or fractures relic of parent rock; and
  - (G) laboratory determinations may be used to supplement field determinations. Split samples shall be made available to the LHD.

History Note: Authority G.S. 130A-335(e); S.L. 2015-147, s.3; S.L. 2023-77, s.12; Eff. January 1, 2024.

#### 15A NCAC 18E .0508 **AVAILABLE SPACE**

(a) Sites shall have available space to allow for the installation of the initial wastewater system and repair area for a system identified or approved in Sections .0900, .1500, or .1700 of this Subchapter. The available space provided shall meet all required setbacks in Sections .0600 or .1200 of the Subchapter and provide access to the wastewater system for operation and maintenance activities. A site with sufficient available space shall be suitable.

- (b) If the site does not have available space for both an initial wastewater system and repair area it shall be unsuitable.
- (c) The repair area requirement of Paragraph (a) of this Rule shall not apply to a lot or tract of land which meets the following:
  - described in a recorded deed or a recorded plat on or before January 1, 1983; (1)
  - (2)insufficient size to satisfy the repair area requirement of Paragraph (a) of this Rule, as determined by the authorized agent:
  - DDF is no more than 480 gallons for a single-family dwelling unit or a single facility; and (3)
  - (4)the proposed facility will generate DSE.

(d) Although a lot or tract of land may be exempt from the repair area requirement under Paragraph (c) of this Rule, the authorized agent shall determine if there is any available space for the repair area. The authorized agent shall determine the maximum feasible repair area available, and that repair area shall be specified on the IP, CA, and OP.

(e) If a site meets any of the following criteria, a repair area shall be required, even if the site is exempt from the repair area requirement of Paragraph (c) of this Rule:

- proposed increase in flow or wastewater strength to an existing facility permitted under the exemption of Paragraph (1)(c) of this Rule; or
- (2)any new initial wastewater system is proposed on a lot or tract of land on which the exemption in Paragraph (c) of this Rule was previously utilized.

(f) Notwithstanding the criteria for when a repair area is required in accordance with Paragraph (e) of this Rule, a site shall remain exempt from the repair area requirements of Paragraph (a) of this Rule when all of the following conditions are met:

- (1)an owner submits an application to the LHD for an increase in flow to an existing facility permitted in accordance with Paragraph (c) of this Rule and the facility DDF remains less than or equal to 480 gpd of DSE;
- (2) there is sufficient available space for the existing system to be modified pursuant to the Rules of this Subchapter;
- (3) the site for the existing system complies with the Rules of this Subchapter and the existing system is not malfunctioning in accordance with Rule .1303(a)(2) of this Subchapter; and (4)
  - the conditions set forth in Paragraph (d) of this Rule are met.

(g) Prior to the issuance of the IP, the proposed dispersal field shall be field located and staked on-contour, as applicable, to verify that initial and repair wastewater systems can be installed in the area delineated. The dispersal field may be installed level but off contour if an authorized agent has determined that there is sufficient vertical separation to a LC along the entire trench length in accordance with Rule .0901(g)(2) of this Subchapter.

(h) The initial and repair area shall not be altered so that the wastewater system specified on the IP, CA, and OP cannot be installed and function as permitted.

(i) A wastewater system certified in accordance with NSF/ANSI Standard 350 or that has data from a two-year field demonstration documenting that the wastewater system meets NSF/ANSI Standard 350 or better may eliminate the requirement for a repair area when installed in Group I soils. The Department shall specify the information required for a wastewater system to be approved with a twoyear field demonstration. A wastewater system that does not have a repair area in accordance with this Paragraph shall only be used to treat DSE, shall meet a TN effluent standard of 20 mg/L, and shall be classified as a Type VI(b) pursuant to Table XXXII of Rule .1301(b) of this Subchapter. Evidence of TN standards shall be provided by applicable NSF/ANSI testing or approval as a TS-II wastewater system in accordance with this Subchapter and included in the IP, CA, or NOI.

# SECTION .0600 – LOCATION OF WASTEWATER SYSTEMS

#### 15A NCAC 18E .0601 LOCATION OF WASTEWATER SYSTEMS

(a) Every wastewater system shall be located the minimum setbacks from the site features specified in Table IX. The setback shall be measured on the ground surface, unless otherwise specified in this Rule, from the nearest wastewater system component sidewall or as otherwise specified in a system specific rule or PIA Approval.

### **TABLE IX.** Minimum setbacks from all wastewater systems to site features

TABLE IX. Minimum setbacks from all wastewater	
Site Features	Setback in feet
Any transient or non-transient non-community water supply well,	100
community well, shared water supply well, well that complies with	
15A NCAC 18A .1700, or water supply spring	
A private drinking water well or upslope spring serving a single	50
family dwelling unit unit, including wastewater systems permitted	
or installed in saprolite	
Any other well or source not listed in this table, excluding	50
monitoring wells	
Surface waters classified WS-I, from ordinary high-water mark	100
Waters classified SA, from mean high-water mark	100
Any Class I or Class II reservoir, from normal water level	100
Lake or pond, from normal water level	50
Any other stream, non-water supply spring, or other surface	50
waters, from the ordinary high-water mark	
Tidal influenced waters, such as marshes and coastal waters, from	50
mean high-water mark	
Permanent stormwater retention basin, from normal water level	50
Any water line, unless the requirements of Paragraph (i) have been	10
met	10
Closed loop geothermal wells	15
	5
Building foundation and deck supports any structural supports	5
requiring a footing or other load bearing construction in the North Carolina Building Code	
	1
Patio, porch, stoop, lighting fixtures, or signage, including	1
supporting structures such as posts or pilings	
Appurtenant structures such as stairs, or landing structures	
designed specifically to be set directly on the ground and do not	
require footings; sidewalks; pavers; lighting fixtures; or signage	1.5
Any basement, cellar, or in-ground swimming pool	15
Buried storage tank or basin, except stormwater	10
Above ground swimming pool and appurtenances that require a	5
building permit	
Top of slope of embankment or cuts of two feet or more vertical	15
height with a slope greater than 50 percent	
Top of slope of embankment or cuts of two feet or more vertical	15
height with a slope greater than 33 percent and less than or equal	
to 50 percent	If the site has suitable soil depth
	that extends for a minimum
	horizontal distance of 15 feet from
	the edge of the dispersal field, no
	minimum setback is required.
Top of slope of embankment or cuts of two feet or more vertical	0
height with a slope less than <u>or equal to</u> 33 percent	Ŭ
Groundwater lowering system, as measured on the ground surface	25
from the edge of the feature	
Downslope interceptor drains and surface water diversions with a	15
vertical cut of more than two feet, as measured on the ground	10
surface from the edge of the feature	
Sarrace from the eage of the feature	

Upslope and sideslope interceptor drains and surface water diversions with a vertical cut of more than two feet, as measured on the ground surface from the edge of the feature	10
A stormwater collection system as defined in 15A NCAC 02H .1002(48), excluding gutter drains that connect to a stormwater collection system, with a vertical cut of more than two feet as measured from the center of the collection system	<del>10</del>
Bio-retention area, injection well, infiltration system, or dry pond	25
Any other dispersal field, except designated dispersal field repair area for project site	20
Any property line	10
Burial plot or graveyard boundary	10
Above ground storage tank from dripline or foundation pad, whichever is more limiting	5
Utility transmission and distribution line poles and towers, including guy wires, unless a greater setback is required by the utility company	5
Utility transformer, ground-surface mounted	5
Underground utilities	5

(b) Wastewater systems may be located closer than 100 feet but never less than 50 feet from water supply wells or an upslope spring for repairs, space limitations, and other site-planning considerations when one of the following conditions is met:

- (1) the well was constructed prior to July 1, 1993, in accordance with 15A NCAC 18A .1720; or
- (2) a variance for a reduced well setback has been issued in accordance with one of the following:
  - (A) 15A NCAC 02C .0118 for a shared water supply well, a wastewater system permitted or installed in saprolite, or for a transient non-community public water supply well; or
    - (B) 15A NCAC 18C .0203(b) for a non-transient non-community public water system.

(c) Wastewater systems shall not be located closer than 100 feet to springs, uncased wells, and ungrouted wells used as a source of drinking water and located downslope from the dispersal field.

(d) Underground utilities maintain a five foot setback and shall not encroach on the wastewater system and repair area.

(e)(d) The reduced setbacks in Table X shall apply to septic tanks and pump tanks if a leak test has been performed at the job site on the septic tank and pump tank in accordance with Rule .0805 of this Subchapter that verifies the tank, pipe penetrations, and riser connections are watertight.

Site Features	Setback in feet
Permanent stormwater retention basin, from normal water level	35
Bio-retention area, injection well, infiltration system, or dry pond	15
Groundwater lowering system, as measured on the ground surface from the edge of the feature	15
Any water line	5
A stormwater collection system as defined in 15A NCAC 02H .1002(48), excluding gutter drains that connect to a stormwater collection system, with a vertical cut of more than two feet as measured from the center of the collection system	5

TABLE X. Reduced setbacks for tanks to some site features

(f)(e) No minimum setback shall be required from a well that has been permanently abandoned in accordance with 15A NCAC 02C .0113 and for which a record of abandonment has been submitted in accordance with 15A NCAC 02C .0114.

(g)(f) Initial and repair dispersal field systems shall not be located under impervious surfaces or areas subject to vehicular traffic unless approved in accordance with G.S. 130A-343 and Section .1700 of this Subchapter.

(h)(g) If a collection sewer supply line or force main is installed under areas subject to vehicular traffic or areas subject to soil disturbance or compaction, one of the following pipe materials shall be used:

- (1) DIP;
- (2) a minimum of Schedule 40 PVC, Polyethylene, or ABS pipe sleeved in DIP;
- (3) a minimum of Schedule 40 PVC, Polyethylene, or ABS pipe sleeved in DOT traffic rated culvert pipe;
- (4) a minimum of Schedule 40 PVC, Polyethylene, or ABS pipe with 30 inches of compacted material provided over the crown of the pipe; or
- (5) other pipe materials may be proposed when designed, inspected, and certified by a PE and approved by the LHD.

(i)(h) In addition to the requirements of Paragraph (a) of this Rule, wastewater systems with a proposed DDF greater than 3,000 gpd, as determined in Rule .0401 of this Subchapter, shall be located the minimum setbacks from the site features in Table XI.

TABLE XI. Minimum setbacks from wastewater systems greater than 3,000 gpd to site features

Feature	Setback in feet
Any Class I or II reservoir or any public water supply source	500
utilizing a shallow, under 50 feet, groundwater aquifer, from	
feature or normal water level	
Any other public water supply source, unless a confined aquifer	200
Any private drinking water well or upslope spring, unless a	100
confined aquifer	
Surface water classified WS- I, from ordinary high-water mark	200
Surface waters classified WS-II, WS-III, B, or SB, from mean	100
high-water mark or ordinary high-water mark	
Waters classified SA, from mean high-water mark	200
Any property line	25

(j)(i) Wastewater systems with a DDF greater than 3,000 gpd that meet the requirements of Rule .0510(f) of this Subchapter may use the setbacks identified in Table IX of this Rule.

(k)(j) Collection sewers sewers, force mains, and supply lines shall be located the minimum setbacks to site features shown in Table IX, unless a different minimum setback is specified in Table XII. When a reduced setback to a collection sewer sewer, force main, or supply line is utilized, the piping requirements for the reduced setback shall be extended to comply with the unreduced setback. The distribution device shall receive the reduced setback when demonstrated to be watertight with an on-site leak test.

TABLE XII. Minimum setbacks from collection sewers sewers, force mains, and supply lines to site features

FeatureSetback in feetAny public water supply source, including wells, springs, and Class I or Class II reservoirs, from feature or normal water level10050, if constructed of or sleeved in Schedule 80 P or DIP with mechanical joints equivalent to wat main standards, and the collection sewer sewer force main, or supply line is leak tested and show to be watertight*Any water supply well excluding those regulated under 15A NCAC 18C5025, if constructed of Schedule 40 pressure rated Water main standards, and the collection sewer sewer, force main, or supply line is leak tested and show to be watertight*	
reservoirs, from feature or normal water level       50, if constructed of of sice ved in Schedule 30 if or DIP with mechanical joints equivalent to wat main standards, and the collection sewer sewer force main, or supply line is leak tested and show to be watertight*         Any water supply well excluding those regulated under 15A NCAC 18C       50         25, if constructed of Schedule 40 pressure rated PVC or DIP with mechanical joints equivalent to water main standards, and the collection sewer	
reservoirs, from feature or normal water level       or DIP with mechanical joints equivalent to water main standards, and the collection sewer sewer force main, or supply line is leak tested and show to be watertight*         Any water supply well excluding those regulated under 15A NCAC 18C       25, if constructed of Schedule 40 pressure rated PVC or DIP with mechanical joints equivalent to water main standards, and the collection sewer	/C
main standards, and the collection sewer sewer force main, or supply line is leak tested and show to be watertight*         Any water supply well excluding those regulated under 15A NCAC 18C         25, if constructed of Schedule 40 pressure rated PVC or DIP with mechanical joints equivalent to water main standards, and the collection sewer	
force main, or supply line         is leak tested and show to be watertight*           Any water supply well excluding those regulated under 15A NCAC 18C         50           25, if constructed of Schedule 40 pressure rated PVC or DIP with mechanical joints equivalent to water main standards, and the collection severe	
to be watertight*         Any water supply well excluding those regulated under 15A NCAC 18C       50         25, if constructed of Schedule 40 pressure rated PVC or DIP with mechanical joints equivalent to water main standards, and the collection severe sever	
regulated under 15A NCAC 18C 25, if constructed of Schedule 40 pressure rated PVC or DIP with mechanical joints equivalent water main standards, and the collection sever	
regulated under 15A NCAC 18C 25, if constructed of Schedule 40 pressure rated PVC or DIP with mechanical joints equivalent water main standards, and the collection sever	
water main standards, and the collection sewer	l
	0
sewer, force main, or supply line is leak tested a	
	ıd
shown to be watertight*	
15, if constructed of Schedule 80 PVC, sleeved	
DIP or Schedule 80 PVC, and the collection sew	
sewer, force main, or supply line is leak tested a	ıd
shown to be watertight*	
Surface waters classified WS-I, WS-II, WS-	
III, B, SA, or SB, from mean high-water mark 10, if constructed of or sleeved in Schedule 80 P	
or ordinary high-water mark or DIP with mechanical joints equivalent to wat	
main standards, and the collection sewer sewer	
force main, or supply line is leak tested and show	/n
to be watertight*	
Any other stream, non-water supply spring, or 10	
other surface waters, from the ordinary high-	
water mark       Tidal influenced waters, such as marshes and       10	
coastal waters, from mean high-water mark	
Closed loop geothermal wells 5	
Any service connection as defined in 15A     5	
NCAC 18C .0102(c)(21)	
Any basement, cellar, or in-ground swimming 10	
pool	
Top of slope of embankment or cuts of two     5	
feet or more vertical height with a slope	
greater than 50 percent	

Interceptor drains and surface water	5
diversions, with a vertical cut of more than	
two feet as measured on the ground surface	
from the edge of the diversion	
Permanent stormwater retention basin, from	10
normal water level	
Bio-retention area, injection well, infiltration	5
system, or dry pond	
Any other dispersal field, except designated	5
dispersal field repair area for project site	
Any property line	5
Burial plot or graveyard boundary	5

\*Pipe materials other than DIP, Schedule 40 pressure rated PVC, or Schedule 80 PVC shall be acceptable when the materials conform to materials, testing methods, and acceptability standards meeting water main standards and when the line has been designed, installed, inspected, and certified by a PE and approved by the LHD.

(1)(k) The minimum setback from water lines to collection sewers sewers, force mains, or supply lines shall be 10 feet, except as follows:

- (1) the water line is laid in a separate trench with the elevation of the bottom of the water line 18 inches above the top of the collection sewer; sewer, force main, or supply line; or
- (2) the water line is laid in the same trench as the collection sewer sewer, force main, or supply line with the water line located on one side of the trench, on a bench of undisturbed earth and with the elevation of the bottom of the water line 18 inches above the top of the collection sewer. sewer, force main, or supply line. The collection sewer sewer, force main, or supply line shall be located the width of the trench from the water line.

(m)(1) Collection sewers sewers, force mains, and supply lines and water lines shall not cross, except as follows:

- (1) 18 inches clear vertical separation is maintained, with the collection sewer sewer, force main, or supply line crossing under the water line; or
- (2) the water line crosses under the collection sewer sewer, force main, or supply line or 18 inches clear vertical separation is not maintained and the following criteria are met:
  - (A) the collection sewer sewer, force main, or supply line is constructed of DIP with joints equivalent to water main standards and extends 10 feet on each side of the point of crossing, with full sections of pipe centered at the point of crossing; and
  - (B) the water line is constructed of ferrous materials with joints equivalent to water main standards and extends a minimum of 10 feet on each side of the point of crossing, with full sections of pipe centered at the point of crossing.

(n)(m) Collection sewers sewers, force mains, and supply lines shall not cross storm drains, except as follows:

- (1) 12 inches clear vertical separation is maintained between the collection sewer sewer, force main, or supply line and storm drain;
- (2) the collection sewer sewer, force main, or supply line is constructed of DIP with mechanical joints or restrained pushon joints equal to water main standards; or
- (3) the collection sewer sewer, force main, or supply line is encased in concrete or DIP for a minimum of five feet on either side of the crossing.

(o)(n) Collection sewers sewers, force mains, and supply lines shall not cross under a stream, except as follows:

- (1) a minimum of 36 inches of separation from the stream bottom is maintained;
- (2) the collection sewer sewer, force main, or supply line is constructed of DIP with mechanical joints or restrained pushon joints equal to water main standards; or
- (3) the collection sewer sewer, force main, or supply line is encased in concrete or DIP for a minimum of 10 feet on either side of the crossing and protected against the normal range of high and low water conditions, including the 100-year flood or wave action.

(p)(o) Collection sewer sewer, force main, or supply line aerial crossings shall be constructed of DIP with mechanical joints or restrained push-on joints equal to water main standards and freeze protected. Pipe shall be anchored for a minimum of 10 feet on either side of the crossing.

(q)(p) If septic tanks, pump tanks, grease tanks, raw sewage lift stations, wastewater treatment plants, sand filters, and other advanced pretreatment systems are located in areas subject to flooding at a frequency greater than a 10-year storm, they shall be designed and installed to be watertight and to remain operable during all flooding events.

History Note: Authority G.S. 130A-334; 130A-335(e) and (f); S.L. 2019-215, s.2; <u>S.L. 2024-49, s.4.22;</u> Eff. January 1, 2024.

# SECTION .0700 – COLLECTION SEWERS, RAW SEWAGE LIFT STATIONS, SEPTIC TANK EFFLUENT PUMP SYSTEMS, AND PIPE MATERIALS

#### 15A NCAC 18E .0701 COLLECTION SEWERS

(a) Collection sewers for wastewater systems with a DDF greater than 3,000 gpd shall be designed and constructed in accordance with the following criteria:

- (1) Building drains and building sewers shall be in accordance with the North Carolina Plumbing Code and approved by the local building inspector.
- (2) Pipe material shall be specified to comply with the applicable ASTM standards based on pipe material.
- (3) Gravity sewers shall be designed to maintain minimum scour velocities of two feet per second with the pipe half full and one foot per second at the peak projected instantaneous flow rate. Force mains shall be sized to obtain a minimum two-foot per second scour velocity at the projected pump operating flow rate.
- (4) Infiltration and exfiltration shall not exceed 100 gpd per inch diameter per mile of gravity sewer pipe or 20 gpd per inch diameter per mile of pressure pipe in force mains and supply lines.
- (5) Collection sewers shall be buried three feet deep, except as provided for in Rule .0601(h)(4) of this Subchapter.
- (6) Ferrous material pipe or other pipe designed and bedded for traffic-bearing loads shall be provided where collection sewers are subject to vehicular traffic.
- (7) Manholes shall be used for gravity collection sewers at any bend, junction, and a maximum of every 425 feet along the collection sewer. Drop manholes shall be required where the inlet to outlet elevation difference exceeds two and one half feet. Manhole lids shall be watertight if located below the 100-year flood elevation, within 100 feet of any public water system source, or within 50 feet of any private water system source or any surface waters classified WS-I, WS-II, WS-III, SA, SB, or B.
- (8) Cleanouts may be used instead of manholes for four-inch and six-inch sewers serving one or two design units, or as otherwise allowed by the North Carolina Plumbing Code. Cleanouts shall be required a maximum of every 100 feet for four or six-inch sewers and at all junctions and bends which exceed 45 degrees, unless otherwise allowed by the North Carolina Plumbing Code.
- (9) Air relief valves shall be provided as needed for force mains when the length exceeds 1,000 feet or for intermediate high points that exceed five feet.
- (10) Collection sewers may require additional ventilation provisions, such as a stand pipe, based on length, size, and location.

(b) STEP systems may be used as an alternative to gravity collection sewers.

History Note: Authority G.S. 130A-335(e), (f), and (f1); <u>S.L. 2024-49, s.4.23;</u> Eff. January 1, 2024.

# 15A NCAC 18E .0702 RAW SEWAGE LIFT STATIONS

(a) Raw sewage lift stations permitted by the LHD for wastewater systems with a DDF greater than 3,000 gpd shall meet all setbacks for wastewater systems in accordance with Table IX of Rule .0601(a) of this Subchapter.

(b) Raw sewage lift stations for wastewater systems with a DDF greater than 3,000 gpd shall meet the following design and construction standards:

- (1) dual pumps shall be provided for stations serving two or more buildings or for a facility with more than six water closets;
- (2) pumps shall be listed by a third-party electrical testing and listing agency, such as Underwriter's Laboratories;
- (3) pumps shall be grinder pumps or solids-handling pumps capable of handling a minimum of three-inch spheres. If the raw sewage lift station serves no more than a single water closet, lavatory, and shower, two-inch solids handling pumps shall be acceptable;
- (4) minimum pump capacity shall be two and one half times the average daily flow;
- (5) raw sewage lift stations serving single buildings shall be designed for pump run times between three to 10 minutes at average daily flow;
- (6) pump station emergency storage capacity and total liquid capacity shall be determined in accordance with Rule .0802 of this Subchapter except for a sealed, watertight chamber serving an individual building, in which case a minimum storage capacity of eight hours shall be required; and
- (7) all applicable requirements for pump tanks and dosing systems as set forth in Rule .0802 and Section .1100 of this Subchapter shall apply to raw sewage lift stations.

(c) A raw sewage lift station that is a sealed, watertight chamber shall meet the setback requirements for collection sewers in Rule .0601(k) of this Subchapter. Sealed, watertight chambers shall be a single prefabricated unit with a sealed top lid, and preformed inlet and outlet pipe openings connected with solvent welds, O-ring seals, rubber boots, stainless steel straps, or equivalent.

History Note: Authority G.S. 130A-335(e), (f), and (f1); <u>S.L. 2024-49, s.4.24;</u> Eff. January 1, 2024.

# 15A NCAC 18E .0703 PIPE MATERIALS

(a) The gravity pipe between a septic tank, gravity distribution device, and the dispersal field shall be a minimum of three-inch Schedule 40 PVC, Schedule 40 polyethylene, or Schedule 40 ABS. ABS, with a minimum fall of 1/8-inch per foot if the installation requirements of Paragraph (b) of this Rule are met.

(b) Three-inch or greater non-perforated polyethylene corrugated tubing, PVC SDR 21 and SDR 26 pressure rated at 160 psi or greater and labeled as compliant with ASTM D2241, PVC SDR 35 gravity sewer pipe rated as compliant with ASTM D3034, or alternative non-perforated pipe materials described in Paragraph (d) of this Rule, may be substituted for Schedule 40 between the distribution device and the dispersal field when the following minimum installation criteria are met:

- (1) the pipe is placed on a compacted, smooth surface free of indentations or clods at a uniform grade, and with an excavation width of one foot;
- (2) the pipe is placed in the middle of the excavation with three inches of clearance between the pipe and the walls;
- (3) a washed gravel or crushed stone envelope is placed in the excavation on both sides of the pipe and to a point two inches above the top of the pipe;
- (4) six inches of soil is placed and compacted over the stone or gravel envelope; and
- (5) earthen dams consisting of two feet of undisturbed or compacted soil are located at both ends of the excavation separating the trench from the distribution device.

(c) All pipe joints from the septic tank to the dispersal field shall be watertight. Solvent cement-joints shall be made in a two-step process with primer manufactured for thermoplastic piping systems and solvent cement conforming to ASTM D2564.

(d) Pipe used for gravity distribution laterals shall be corrugated plastic tubing complying with ASTM F667 or smooth-wall plastic pipe complying with ASTM D2729 or ASTM F810. The pipe shall be marked as complying with ASTM standards. The corrugated tubing or smooth-wall pipe shall have three rows of holes, each hole between one-half inch and three-fourths inches in diameter and spaced longitudinally approximately four inches on centers. The rows of holes may be equally spaced 120 degrees on centers around the pipe periphery, or three rows may be located in the lower portion of the tubing, the outside rows being approximately on 120-degree centers. The holes may be located in the same corrugation or staggered in adjacent corrugations. Other types of pipe may be used for laterals provided the pipe satisfies the requirements of this Rule and is approved by the Department.

(e) Pump discharge piping, including the force main to the next component in the wastewater system, shall be of Schedule 40 PVC or stronger material and pressure rated for water service at a minimum of 160 psi or two times the maximum operating pressure, whichever is greater. The pipe shall meet ASTM D1784, ASTM D1785, and ASTM D2466.

(f) Pipe materials other than those identified in this Rule may be proposed when designed and certified by a PE, including any installation and testing procedures. Gravity pipe materials shall be shown to comply with the requirements of Paragraphs (a), (b), and (c) of this Rule. Alternative pressure rated pipe materials shall be constructed of PVC, polyethylene, or other pressure rated pipe and conform to applicable ASTM standards for pipe material and methods of joining. The proposed pipe shall be installed per ASTM D2774. Installation testing shall include a hydrostatic pressure test similar to pressure testing required for water mains for any line exceeding 500 feet in length and shall comply with the requirements of Rule .0701(a)(4) of this Section.

History Note: Authority G.S. 130A-335(e), (f), and (f1); <u>S.L, 2024-49, s.4.25;</u> Eff. January 1, 2024.

#### SECTION .0800 - TANK CAPACITY, LEAK TESTING, AND INSTALLATION REQUIREMENTS

#### 15A NCAC 18E .0801 SEPTIC TANK CAPACITY REQUIREMENTS

(a) Minimum liquid capacities for septic tanks shall be in accordance with the following:

- (1) The minimum capacity of any septic tank shall be 1,000 gallons unless otherwise provided for in this Rule.
- (2) The minimum capacity of any septic tank serving an individual dwelling unit with five bedrooms or less shall be sized as set forth in Table XIV.

Number of bedrooms	Minimum liquid capacity in gallons
4 or less	1,000
5	1,250

TABLE XIV. Minimum	ptic tank liquid capacity for dwe	ling units
	prie tunk negata capacity for ane	ining units

- (3) Septic tanks for dwelling units greater than five bedrooms, multiple dwelling units, places of business, or places of public assembly shall be sized in accordance with Table XV.
- (4) The minimum septic tank capacity serving two or more dwelling units shall be <u>sized in accordance with Table XV.</u> 1,500 gallons. based on the DDF of 120 gpd per bedroom or 60 gpd per person.

Design daily flow in gpd (Q)	Minimum septic tank liquid capacity (V) calculation in gallons
$Q \le 600$	V = 2Q
600 < Q < 1,500	V = 1.17Q + 500
$1,500 \le Q \le 4,500$	V = 0.75Q + 1,125

TABLE XV. Septic tank capacity for facilities not listed in Table XIV

Q > 4,500	V = Q

(5) Septic tanks for RWTS and PIA Systems shall be sized in accordance with the RWTS or PIA Approval, pursuant to Sections .1500 and .1700 of this Subchapter.

(b) The minimum liquid capacity requirements of Paragraph (a) of this Rule shall be met by use of a single two compartment tank or by two tanks installed in series. The tanks in series may be constructed with or without a baffle wall. Each tank shall have a minimum liquid capacity of 1,000 gallons.

(c) When a grinder pump or sewage lift pump is installed prior to the septic tank, the required septic tank liquid capacity as set forth in this Rule shall be doubled. The minimum liquid capacity may be met by installing two or more septic tanks in series, each tank containing two compartments. The minimum liquid capacity of each tank shall be 1,000 gallons. The installation of two septic tanks in series is not required if the grinder pump or sewage lift pump discharges into 10 feet of four-inch pipe, as specified by the North Carolina Plumbing Code.

(d) The Department shall review other septic tanks designed to receive wastewater from grinder pumps or sewage lift pumps if designed by a PE to ensure that effluent discharged from the septic tank meets DSE as set forth in Table III of Rule .0402(a) of this Subchapter.

(e) An effluent filter approved in accordance with Rule .1404 of this Subchapter shall be in the outlet of the final compartment of the septic tank.

(f) When two or more tanks are used in series in accordance with Paragraphs (b) or (c) of this Rule, the following conditions shall be met:

- (1) the outlet of the initial tank shall consist of an outlet sanitary tee extending down 25 to 50 percent of the liquid depth; and
- (2) an approved effluent filter shall be in the outlet of the final compartment.

History Note: Authority G.S. 130A-334; 130A-335(e), (f), and (f1); <u>S.L. 2023-77, s.14; S.L. 2024-49, s.4.26;</u> Eff. January 1, 2024.

# 15A NCAC 18E .0805 TANK LEAK TESTING AND INSTALLATION REQUIREMENTS

(a) All tanks installed under the following conditions shall be leak tested:

- (1) when a SWC is present within four feet of the elevation of the top of a mid-seam pump tank;
- (2) with advanced pretreatment when required in the RWTS or PIA Approval;
- (3) when required in the approved plans and specifications for a wastewater system designed by a <u>PE; PE or AOWE;</u>
- (4) when the tank is constructed in <del>place</del>; <u>place at the jobsite by a person not approved by the Department as a tank</u> manufacturer using bricks, blocks, or poured in place concrete; or
- (5) as required by the authorized agent based upon site or system specific conditions, such as misaligned seams, exposed reinforcement, or damage observed that may have occurred during transport or installation.

(b) Tanks subject to leak testing in accordance with Paragraph (a) of this Rule shall be leak tested using either a hydrostatic test procedure or vacuum test procedure as follows:

- (1) The operational procedures to be followed for the hydrostatic test are:
  - (A) fill tank with water to the <u>underside of the top of the tank or, for corrugated tanks, to the bottom of the upper</u> most corrugation that forms the top of the tank; outlet invert or pipe, as applicable;
  - (B) allow the tank to sit for one hour;
  - (C) tank shall be approved if the water level drops less than or equal to one-eighth inch in one hour;
  - (D) if a leak is detected, the tank may be repaired in accordance with the tank manufacturer's written instructions, refilled, and retested;
  - (E) surface wetness or condensation shall not be considered an active water leak; and
  - (F) the tank manufacturer or installer is allowed one attempt to retest the tank before the authorized agent can deny the tank for use in the installation based on failure to pass the leak test.
- (2) The operational procedures to be followed for the vacuum test are:
  - (A) temporarily seal inlet and outlet pipes and access openings;
  - (B) using calibrated equipment, draw a vacuum on the empty tank to a negative pressure of two and one half inches of mercury;
  - (C) hold the vacuum for five minutes and re-measure and record the ending negative pressure inside the tank;
  - (D) no bracing or internal support that is not part of the approved tank shall be allowed;
  - (E) tank shall be approved if the difference between the starting negative pressure and the ending negative pressure is less than or equal to one-fifth inch;
  - (F) if a leak is detected, the tank may be repaired in accordance with the tank manufacturer's written instructions and retested;
  - (G) the tank manufacturer or installer is allowed one attempt to retest the tank before the authorized agent can deny the tank for use in the installation based on the failure to pass the leak test; and
  - (H) all tank openings shall be un-sealed after the vacuum test is completed.
- (3) Instead of the operational procedures set out in Paragraphs (b)(2)(B) and (b)(2)(E) of this Rule, a tank manufacturer may choose to sue a negative pressure of five inches of mercury held for two minutes with the tank approved if the

difference between the starting negative pressure and the ending negative pressure if less than or equal to two-fifths of mercury.

(c) The LHD shall document the observation of the leak testing.

 $\frac{(e)(d)}{(d)}$  Tanks unable to pass a leak test or be repaired to pass a leak test shall be removed from the site and the imprint described in Rule .1402(d)(15) or (e)(8) of this Subchapter marked over.

(d)(e) The septic tank outlet pipe shall be inserted through the outlet pipe penetration boot, creating a watertight joint, and extending a minimum of two feet beyond the septic tank outlet. The pump tank outlet pipe shall be inserted through the outlet pipe penetration boot, creating a watertight joint, or through another watertight joint, such as a rubber grommet, in the pump tank riser.

(e)(f) The septic tank outlet pipe and pump tank outlet pipe shall be placed on undisturbed soil soil, compacted soil, or bedded in accordance with Rule .0703(b) of this Subchapter to prevent differential settling of the pipe. The <u>pump tank outlet</u> pipe shall be level for a minimum of two feet after exiting the tank.

(f)(g) The tank shall be installed level. A tank is considered level if the difference between the front and back is plus or minus one inch and the difference from side to side is plus or minus one inch. The tank excavation, bedding, backfill, and compaction shall be in accordance with the tank manufacturer's installation requirements, specifications, and the tank approval.

(g)(h) The tank excavation shall be separated from the dispersal system by at least two feet of undisturbed soil. Piping from the tank to the next component shall be placed on undisturbed soil, compacted soil, or bedded using sand, gravel, stone, or other aggregate.

(h)(i) Effluent filters and risers shall be installed in accordance with the design and construction criteria of Rule .1402(b) and (c) of this Subchapter.

(i)(j) Any system serving a facility with a DDF greater than 3,000 gpd shall have access manholes installed on the tank and extending at a minimum to finished grade. The access manholes shall be designed and maintained to prevent surface water inflow and sized to allow access for routine inspections, operation, and maintenance.

History Note: Authority G.S. 130A-335(e), (f), and (f1); <u>S.L. 2023-77, s.15; S.L. 2024-49, s.4.27;</u> Eff. January 1, 2024.

# SECTION .0900 – SUBSURFACE DISPERSAL

# 15A NCAC 18E .0901 GENERAL DESIGN AND INSTALLATION CRITERIA FOR SUBSURFACE DISPERSAL SYSTEMS

(a) Wastewater systems shall be used on sites classified suitable in accordance with Rule .0509 of this Subchapter. The sizing and siting criteria in this Rule shall be based on soil receiving DSE. The site shall meet the following minimum criteria:

- (1) 12 inches of naturally occurring soil between the infiltrative surface and any LC; and
- (2) 18 inches of separation between the infiltrative surface and any SWC if more than six inches of separation consists of Group I soils.

(b) If any part of the trench or bed media extends above the naturally occurring soil surface, the system shall be a fill system and shall meet the requirements of Rule .0909 of this Section.

(c) The LTAR shall be determined in accordance with the following:

- (1) Tables XVII and XVIII shall be used, as applicable;
- (2) the LTAR shall be assigned based upon soil textural class or saprolite textural class, as applicable, structure, consistence, SWC, depth, percent coarse rock, landscape position, topography, and system type;
- (3) LTARs determined from Table XVII shall be based on the soil textural class of the most limiting, naturally occurring soil horizon to a depth of 12 inches below the infiltrative surface or 18 inches to any SWC if more than six inches of the separation consists of Group I soils;
- (4) LTARs determined from Table XVIII shall be based on the saprolite textural class of the most limiting, naturally occurring saprolite to a depth of 24 inches below the infiltrative surface, or less than 24 inches if combined with soil in accordance with Rule .0506(b) of this Subchapter; and
- (5) for facilities that generate HSE as specified in Rule .0401(h) of this Subchapter or a facility with a full kitchen, the LTAR shall not exceed the mean rate for the applicable Soil Group.

Soil Group	USDA Soil Textural Class		LTAR in gpd/ft <sup>2</sup>
Ι	Sands	Sand	0.8 - 1.2
		Loamy Sand	
II	Coarse Loams	Sandy Loam	0.6 - 0.8
		Loam	
III	Fine Loams	Sandy Clay Loam	0.3 - 0.6
		Silt Loam	
		Clay Loam	
		Silty Clay Loam	
		Silt	

TABLE XVII. LTAR for wastewater systems based on Soil Group and texture class

IV	Clays	Sandy Clay	0.1 - 0.4
		Silty Clay	
		Clay	

TABLE XVIII. LTAR for wastewater systems in saprolite based on Saprolite Grou	p and texture class
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Saprolite Group	Saprolite Textural Class		LTAR in gpd/ft <sup>2</sup>
Ι	Sands	Sand	0.6 - 0.8
		Loamy Sand	0.5 - 0.7
II	Loams	Sandy Loam	0.4 - 0.6
		Loam	0.2 - 0.4
III	Fine Loams	Silt Loam	0.1 - 0.3
		Sandy Clay	0.05 - 0.15
		Loam*	

\* Sandy clay loam saprolite can only be used with advanced pretreatment in accordance with Section .1200 of this Subchapter.

(d) The minimum required infiltrative surface area and trench length shall be calculated in accordance with the following:

- (1) the minimum required infiltrative surface area shall be calculated by dividing the DDF by the LTAR;
  - (2) the minimum trench length shall be calculated by dividing the minimum required infiltrative surface area by the equivalent trench width. The following equation shall be used to calculate the minimum trench length required:

Where	TL TI	=	(DDF / LTAR) / ETW trench length, in feet
where	1L DDF	_	design daily flow, in gpd
	LTAR	=	in gpd/ft <sup>2</sup>
	ETW	=	equivalent trench width, in feet;

- (3) the area occupied by step-downs, drop boxes, and supply lines shall not be part of the minimum required infiltrative surface area;
- (4) the total trench length required for trench products other than conventional gravel shall be as follows:
  - (A) for trench products identified in Section .0900 of this Subchapter, the minimum line length shall be calculated in accordance with this Section; or
  - (B) for trench products approved under Section .1700 of this Subchapter, the minimum line length shall be calculated in accordance with the PIA Approval; and
- (5) when HSE is proposed to be discharged to a dispersal field with no advanced pretreatment <u>as required in Rule</u> .0402(b)(1) of this Subchapter or has not been reclassified as DSE in accordance with Rule .0402(c) of this Subchapter, a licensed professional, if required in G.S. 89C, 89E, or 89F, shall calculate the adjusted LTAR in accordance with Rule .0402(b)(2) of this Subchapter.
- (e) Any dispersal field where cover is required above the naturally occurring soil surface shall not be installed on slopes greater than 30 percent.

(f) Soil cover above the original grade shall be placed over the entire dispersal field and shall extend laterally five feet beyond the trenches. On level sites, the final grade of the dispersal field shall be crowned at one-half percent grade as measured from the centerline of the dispersal field.

(g) Wastewater system installation shall be in accordance with the following criteria:

- (1) a device that measures elevation, such as an engineer's level or laser level shall be used for the following:
  - (A) staking, flagging, or marking on the ground surface the location of trenches on site before installation begins;
  - (B) installation of the trenches; and
  - (C) verification of elevations, excavations, and installation of other system components;
- (2) trenches shall be installed with 12 inches of naturally occurring suitable soil between the infiltrative surface and any unsuitable LC. If the vertical separation between the infiltrative surface and any SWC is less than 18 inches, and if more than six inches of the separation consists of Group I soils, a pressure dispersal system shall be required;
- (3) the trenches shall follow the ground contour. Trenches may be installed level but off contour if an authorized agent has determined that there is sufficient vertical separation to a LC along the entire trench length in accordance with Subparagraph (2) of this Paragraph;
- (4) the lateral shall be centered horizontally in the trench;
- (5) the type and placement of soil cover shall be approved by the authorized agent in accordance with this Subparagraph. The cover material shall be free of trash, debris, or large clods that do not break apart. The system can be installed utilizing native backfill unless otherwise specified in this Section or the PIA Approval:
- (6) final soil cover over the dispersal field shall be a minimum of six inches deep after settling. The finished grade over the tanks and dispersal field shall be sloped to shed surface water;

- (7) surface water runoff, including stormwater, gutter drains, or downspouts, shall be diverted away from the wastewater system. No depressions shall be allowed over the dispersal field area;
- (8) Schedule 40 PVC or other pipe approved pursuant to Section .0700 of this Subchapter may be used as needed to connect sections of trench and overcome site limitations. The trench bottom area where solid piping is installed shall not be included as part of the minimum required infiltrative surface area;
- (9) gravity effluent distribution components including distribution boxes, drop boxes, and flow diversion devices shall be watertight, corrosion resistant, constructed to withstand active and passive loads, and their installation shall meet the following criteria:
  - (A) separated by a minimum of two feet of undisturbed soil from the septic tank and trench(es);
  - (B) placed level on a solid foundation of undisturbed soil, pea gravel, or concrete to prevent differential settling of the component; and
  - (C) backfilled by hand to minimize disturbance;
- (10) when parallel distribution is used to distribute effluent to the trenches, the installer shall demonstrate to the authorized agent during the final inspection that the distribution devices perform as designed;
- (11) serial and sequential distribution shall be approved by the authorized agent when the step-down or drop box in an individual trench is constructed to allow full utilization of the upstream trench prior to overflowing to the next downslope trench in accordance with the following criteria:
  - (A) step-downs shall be constructed of a minimum of two feet of undisturbed soil, bedding material, or concrete and the effluent shall be conveyed over the step-down through Schedule 40 PVC or other pipe approved in accordance with Rule <u>.0703 .0703(f)</u> of this Subchapter. <u>Nonperforated corrugated polyethylene tubing may be used on sites with slopes greater than 25 percent.</u> The installer shall demonstrate that the step-downs perform as designed. The authorized agent shall approve the step-downs when the installation and elevations have been verified in accordance with the CA; or
  - (B) drop boxes shall be separated from the trench by a minimum of two feet of undisturbed soil and constructed to allow for full utilization of the upstream trench prior to overflowing to the next lower drop box. The installer shall demonstrate that the drop boxes perform as designed. The authorized agent shall approve the drop boxes when the installation and elevations have been verified in accordance with the CA; and
- (12) trench products other than conventional gravel shall be installed as follows:
  - (A) for trench products identified in Section .0900, the trench products shall be installed in accordance with this Section; or
  - (B) for trench products approved under Section .1700 of this Subchapter, the trench products shall be installed in accordance with their PIA Approval.

(h) Alternating dual dispersal fields shall only be used with DSE in Soil Groups III and IV. Alternating dual dispersal fields shall be approved when designed and installed in accordance with Paragraph (g) of this Rule and the following:

- (1) both initial and repair dispersal fields shall be installed at the same time;
- (2) initial and repair dispersal fields of the same system type are each sized at a minimum of 75 percent of the total trench length required;
- (3) the initial and repair dispersal fields shall be separated by an effluent flow diversion valve(s);
- (4) diversion valve(s) shall be resistant to 500 pounds crushing strength and corrosion resistant;
- (5) effluent flow diversion valves shall be installed below finished grade in a valve box and be accessible and operable from the ground surface; and
- (6) trench products approved under Section .1700 of this Subchapter shall be installed in accordance with their PIA Approval.

History Note: Authority G.S. 130A-335(e), (f), and (f1); <u>S.L. 2024-49, s.4.28;</u> Eff. January 1, 2024.

# 15A NCAC 18E .0902 CONVENTIONAL WASTEWATER SYSTEMS

(a) A conventional wastewater system shall consist of a septic tank and a gravity distribution dispersal field. In addition to the requirements set forth in Rule .0901 of this Section, this Rule shall apply to conventional wastewater systems as defined in G.S. 130A-343.

(b) In addition to the installation requirements set forth in Rule .0901(g) of this Section, the following shall apply:

- (1) trenches shall be constructed level in all directions with a plus or minus one-half inch tolerance from side-to-side and the maximum fall in a single trench not to exceed one-fourth inch in 10 feet as determined by a device that measures elevation, such as an engineer's level or laser level;
- (2) trenches shall be located not less than three times the trench width on centers. The minimum spacing for trenches is six feet on center;
- (3) trench widths shall be at least two feet, but no more than three feet, and trench depth shall not exceed 36 inches on the downslope side of the trench, except as approved by an authorized agent;
- (4) aggregate used in trenches shall be clean, washed gravel or crushed stone and graded or sized in accordance with size numbers 4, <u>467M</u>, 5, or <u>6</u>, <u>57</u>, or <u>67</u> of ASTM D448. The aggregate shall be distributed uniformly across the

infiltrative surface and over the pipe and placed 12 inches deep with a minimum of six inches below the pipe and two inches over the pipe; and

(5) the laterals shall meet the requirements of Rule .0703(d) of this Subchapter.

History Note: Authority G.S. 130A-335(e) and (f); 130A-343; <u>S.L. 2024-49, s.4.29;</u> Eff. January 1, 2024.

#### 15A NCAC 18E .0903 BED SYSTEMS

- (a) This Rule shall apply to bed systems receiving DSE.
- (b) Bed systems shall be limited to 600 gpd unless approved for a greater DDF in accordance with a PIA Approval.
- (c) Sites for bed systems shall meet the following criteria:
  - (1) soil texture is Group I, II, or <del>III;</del> <u>III to a depth of 48 inches below the naturally occurring soil surface or to a depth of 12 inches below the infiltrative surface, whichever is deeper;</u> and
  - (2) design options for the site are limited by topography or available space.

(d) The number of square feet of infiltrative surface area required shall be increased by 50 percent over that required for a trench system as calculated in accordance with Rule .0901(d) of this Section.

(e) In addition to the installation requirements set forth in Rule .0901(g) of this Section, the following shall apply:

- (1) the bottom of the bed shall be excavated level, plus or minus one-half inch, in all directions;
- (2) laterals shall be one and one-half feet from the side of the bed;
- (3) laterals shall be placed on three-foot centers;
- (4) aggregate used shall comply with the requirements of Rule .0902(b)(4) of this Section;
- (5) products approved under Section .1700 of this Subchapter shall be installed in accordance with their PIA Approval;
- (6) the gravel surface shall be covered by an approved geo-textile fabric capable of preventing the downward movement of soil particles while allowing the movement of liquids and gases; and
- (7) when pressure dispersal is used, the lateral design criteria shall meet the minimum requirements of Rules .0907(e) or .0908(d) of this Section or in accordance with a PIA Approval.

History Note: Authority G.S. 130A-335(e), (f), and (f1); <u>S.L. 2023-77, s.16;</u> Eff. January 1, 2024.

#### 15A NCAC 18E .0904 LARGE DIAMETER PIPE SYSTEMS

(a) LDP systems consist of laterals composed of corrugated, polyethylene tubing encased in a nylon and polyester blend filter wrap that are installed in trenches in the dispersal field. The laterals shall be one of the following:

- (1) eight-inch inside diameter with a 10-inch outside diameter; or
- (2) 10-inch inside diameter with a 12-inch outside diameter.

(b) LDP systems shall only be used with DSE. not be used with food service establishments or other facilities where the fats, oils, and grease exceed the limit of DSE when measured as it enters the dispersal product.

(c) LDP pipe, filter wrap, and fittings shall meet the following criteria:

- (1) pipe and fittings shall comply with the requirements of ASTM F667;
- (2) the corrugated pipe shall have two rows of holes, each hole between three-eighths inch and one-half inch in diameter, located 120 degrees apart along the bottom half of the pipe with each hole 60 degrees from the bottom center line, and staggered so that one hole is present in the valley of each corrugation;
- (3) pipe shall be marked with a visible top location indicator, 120 degrees away from each row of holes;
- (4) corrugated pipe shall be covered with filter wrap at the factory;
- (5) filter wrap shall be spun, bonded, or spunlaced nylon, polyester, or nylon/polyester blend filter wrap meeting the minimum requirements in Table XIX; and
- (6) the LDP with filter wrap shall be encased in a black polyethylene sleeve prior to installation in the trench to prevent physical damage and ultraviolet radiation deterioration of the filter wrap.

Property	Value
Unit Weight	1.0 ounce per square yard
Sheet Grab Tensile Strength	Machine Direction: 23 pounds
Trapezoid Tear Strength	Machine Direction: 6.2 pounds
Mullen Burst Strength	40 psi or 276 kilopascals
Frazier Air Permeability	500 cubic feet per minute per square foot at pressure differential of one-half inch of water

Table XIX. Minimum filter wrap requirements for LDP

(d) The requirements of Rule .0901 of this Section shall apply to LDP systems except as follows:

- (1) the LTAR determined in accordance with Rule .0901(c) of this Section shall not exceed 0.8 gpd/ft<sup>2</sup>; and
- (2) to calculate the minimum trench length in accordance with Rule .0901(d) of this Section, an equivalent trench width of two feet shall be used for eight-inch LDP and two and one-half feet shall be used for 10-inch LDP.
- (e) In addition to the requirements set forth in Rule .0901(g) of this Section, LDP system installations shall comply with the following:
  - (1) trenches for eight-inch LDP shall be a minimum of 10 inches and a maximum of 18 inches wide. Trenches for teninch LDP shall be a minimum of 12 inches and a maximum of 24 inches wide;
  - (2) the infiltrative surface and pipe shall be level with a maximum fall of one inch in 100 feet;
  - backfill shall have no more than 10 percent by volume of fibrous organics, building rubble, rocks, large clods, or other debris and shall be Soil Groups I, II, or III; debris;
  - (4) the LDP shall be connected to the collection sewer or a stepdown pipe using an offset adapter to create a mechanical joint; and
  - (5) the minimum on center spacing for eight-inch LDP shall be five feet and for 10-inch LDP shall be six feet.

History Note: Authority G.S. 130A-335(e) and (f); <u>S.L. 2024-49, s.4.30;</u> Eff. January 1, 2024.

# 15A NCAC 18E .0905 PREFABRICATED PERMEABLE BLOCK PANEL SYSTEMS

(a) PPBPS utilize both horizontal and vertical air chambers in a 16-inch PPBPS and are constructed to promote downline and horizontal distribution of effluent. PPBPS systems shall only be used with DSE. not be used with food service establishments or other facilities where the fats, oils, and grease exceed the limit of DSE when measured as it enters the dispersal product.

(b) The requirements of Rule .0901 of this Section shall apply to PPBPS systems except as follows:

- (1) the LTAR determined in accordance with Rule .0901(c) of this Section shall not exceed 0.8 gpd/ft<sup>2</sup>; and
- (2) to calculate the minimum trench length in accordance with Rule .0901(d) of this Section, an equivalent trench width of six feet shall be used.

(c) In addition to the requirements set forth in Rule .0901(g) of this Section, PPBPS system installations shall comply with the following and the manufacturer's specifications:

- (1) PPBPS trenches shall be located a minimum of eight feet on center or three times the trench width; width, whichever is greater; and
- (2) trench sidewalls shall be raked in Group IV soils.

(d) When used in bed and fill bed, fill, or sand lined trench systems, PPBPS shall use the equivalent trench width of six feet to calculate the minimum trench or lateral length required. required, unless otherwise instructed by the manufacturer on a case-by-case basis.
 (e) When used in sand lined trench systems, PPBPS shall use the equivalent trench width of three feet to calculate the minimum trench length required.

History Note: Authority G.S. 130A-335(e) and (f); <u>S.L. 2023-63, s.16; S.L. 2024-49, s.4.31;</u> Eff. January 1, 2024.

# 15A NCAC 18E .0906 SAND LINED TRENCH SYSTEMS

(a) Sand lined trench systems receiving DSE may be used on sites originally classified unsuitable due to SWC, soil morphology, restrictive horizon, or soil depth that may be reclassified as suitable in accordance with this Rule when there is a DDF less than or equal to 1,500 gpd.

- (b) Sand lined trench systems with advanced pretreatment shall comply with Rule .1205 of this Subchapter.
- (c) The soil and site shall meet the following criteria:
  - (1) the texture of the receiving permeable horizon is sand, loamy sand, sandy loam, loam, or silt loam;
  - (2) the structure of the receiving permeable horizon is classified suitable;
  - (3) the moist consistence of the receiving permeable horizon is loose, very friable, friable, or firm;
  - (4) if the receiving permeable horizon has zones of heavier textured materials, these zones are discontinuous with an average thickness not exceeding one-third of the required thickness of the receiving permeable horizon;
  - (5) the naturally occurring receiving permeable horizon shall be less than or equal to 60 inches below the naturally occurring soil surface. If the receiving permeable horizon is greater than 60 inches below the naturally occurring soil surface, advanced pretreatment shall be used in accordance with Rule .1205 of this Subchapter;
  - (6)(5) artificial drainage shall be provided, as needed, to maintain the following minimum vertical separation from the infiltrative surface to a SWC:
    - (A) 18 inches with gravity or pressure dosed gravity distribution; or
    - (B) 12 inches with pressure dispersal; and
  - (7)(6) the minimum required thickness of the receiving permeable horizon shall be determined by the texture of that horizon as follows:
    - (A) sand or loamy sand texture requires a minimum thickness of one foot;
    - (B) sandy loam or loam texture requires a minimum thickness of two feet; or
    - (C) silt loam texture requires a minimum thickness of three feet.

(d) If a groundwater lowering system is required to comply with the minimum vertical separation in Paragraph (c)(6) of this Rule to a SWC that is not related to lateral water movement, design plans and specifications shall be prepared by a licensed professional if required in G.S. 89C, 89E, or 89F. The groundwater lowering system shall:

- (1) extend into the receiving permeable horizon;
- (2) have an outlet with location and elevation that allows for free discharge of groundwater as required for the groundwater lowering system to be functional. The outlet location and elevation shall be shown on the artificial drainage system plan with relative water level elevations and wastewater system site elevations labeled; and
- (3) all groundwater lowering system components are integral to the wastewater system and subject to ownership and control requirements of Rule .0301(b) and (c) of this Subchapter.

(e) The LTAR shall be determined in accordance with Table XX for sand-lined trench systems. The minimum trench length shall be calculated in accordance with Rule .0901(d) of this Section, except that the ETW shall be equal to the installed trench width. The LTAR shall be based on the lesser of the following:

- (1) LTAR set forth in Table XX based on the most hydraulically limiting, naturally occurring soils overlying the permeable receiving horizon; or
- (2) 10 percent of the in-situ Ksat of the receiving permeable horizon.

TABLE XX. LTAR for sand lined trench systems based on the most hydraulically limiting, naturally occurring soils overlying the
permeable receiving horizon

Soil Group	Texture of Most Hydraulically Limiting Overlying Soil Horizon	Distribution Type	LTAR in gpd/ft <sup>2</sup>
Ι	Sands	Gravity or Pressure Dosed Gravity	0.7 - 0.9
		Pressure Dispersal	0.8 - 1.2
II	Coarse Loams	Gravity or Pressure Dosed Gravity $0.5 - 0.7$	
		Pressure Dispersal	0.6 - 0.8
III	Fine Loams	Gravity or Pressure Dosed Gravity	0.2 - 0.4
		Pressure Dispersal	0.3 - 0.6
IV	Clays	Gravity or Pressure Dosed Gravity $0.1 - 0.2$	
		Pressure Dispersal	0.15 - 0.3

(f) There shall be no reduction in trench length compared to a conventional wastewater system when Accepted or Innovative gravelless trench product is used. Trench length for trench dispersal products approved with a specific dispersal field reduction in area or trench length when receiving DSE in accordance with this Subchapter or a PIA Approval shall be calculated in accordance with this Subchapter or PIA Approval.

(g) A special site evaluation in accordance with Rule .0510 of this Subchapter shall be required for the following conditions to field verify the LTAR:

(1) the texture of the receiving permeable horizon is sandy loam or loam and the system DDF is greater than 600 gpd; or

(2) the texture of the receiving permeable horizon is silt loam.

(h) In addition to the requirements set forth in Rule .0901(g) of this Section, sand lined trench system installations shall comply with the following:

- (1) gravity trenches shall have a maximum width of three feet and a minimum width of one and a half feet;
- (2) trenches shall be located not less than three times the trench width on center. The minimum spacing for trenches shall be five feet on center;
- (3) the sand lined trenches shall be constructed to extend into the naturally occurring receiving permeable horizon;
- (4) the infiltrative surface shall be no deeper than 24 inches below finished grade. The top of the trench media shall be at or below the naturally occurring soil surface. Drip tubing shall be installed a minimum of six inches below the natural grade;
- (5) soil used to line the trench shall be sand in texture. The installer shall provide written laboratory verification of the media textural classification and quality when requested by the LHD based on a visual inspection of the sand used during installation. When laboratory analysis is required, the material shall be clean, uncoated fine, medium, or coarse sand with a minimum of 90 percent in sizes ranging from 0.1 to 2.0 millimeters, with no more than one percent smaller than 0.074 millimeters or a No. 200 Sieve;
- (6) pressure dosed gravity distribution or pressure dispersal shall be used when the total dispersal field line length exceeds 750 linear feet in a single system;
- (7) pressure dispersal shall be used when the total dispersal field line length exceeds 1,200 linear feet in a single system;
- (8) when pressure dispersal is used, the pressure dispersal network shall be designed in accordance with Rules .0907(e) or .0908(f) of this Section, except that the trench width shall comply with this Paragraph. The total line length shall be calculated based on infiltrative surface area;
- (9) drip dispersal systems in sand lined trenches shall require multiple runs per trench of drip tubing with emitters as follows:
  - (A) a minimum of two runs within a trench between one and one half and two feet wide; and

(B) a minimum of three runs within a trench between two and three feet wide.

The drip tubing shall be uniformly spaced across the trench with the tubing six inches from the trench sidewalls. Drip tubing shall be covered by a minimum of six inches of sand lined trench media meeting the requirements of Subparagraph (5) of this Paragraph. Drip dispersal systems shall comply with the requirements of Section .1600 of this Subchapter and this Rule;

(10) finished grade shall provide for positive surface drainage away from all system components, with the dispersal field crowned at one-half percent as measured from the centerline of the dispersal field. The finished grade requirements shall be made a condition of the CA; and

(11) trench products approved under Section .1700 of this Subchapter shall be installed in accordance with PIA Approval.
 (i) Other sand lined trench systems may be approved on a site-specific basis in accordance with Rule .0509(c) of this Subchapter.

History Note: Authority G.S. 130A-335(e) and (f); <u>S.L. 2023-77, s.17; S.L. 2024-49, s.4.32;</u> Eff. January 1, 2024.

## 15A NCAC 18E .0907 LOW PRESSURE PIPE SYSTEMS

(a) LPP systems utilize a network of small diameter pipes with three feet to six feet pressure head to distribute effluent across the entire dispersal field. Any subsurface dispersal system listed in this Section may incorporate LPP dispersal.

(b) LPP systems with advanced pretreatment shall comply with Rules .1202, .1203, .1205, or .1206 of this Subchapter.

- (c) The LTAR shall be determined as follows:
  - (1) Tables XXI and XXII shall be used to determine the LTAR for LPP systems, as applicable;
  - (2) the LTAR determined from Table XXI shall be based on the soil textural class of the most limiting, naturally occurring soil horizon to a depth of 12 inches below the infiltrative surface;
  - (3) the LTAR determined from Table XXII shall be based on the saprolite textural class of the most limiting, naturally occurring saprolite to a depth of 24 inches below the infiltrative surface, or less than 24 inches if combined with soil in accordance with Rule .0506(b) of this Subchapter; and
  - (4) for facilities that generate HSE as specified in Rule .0401(h) of this Subchapter or a facility with a full kitchen, the LTAR shall not exceed the mean rate for the applicable Soil Group.

Soil Group	USDA Soil Textural Class		LTAR in gpd/ft <sup>2</sup>
Ι	Sands	Sand Loamy Sand	0.4 - 0.6
II	Coarse Loams	Sandy Loam Loam	0.3 – 0.4
III	Fine Loams	Sandy Clay Loam Silt Loam Clay Loam Silty Clay Loam Silt	0.15 - 0.3
IV	Clays	Sandy Clay Silty Clay Clay	0.05 - 0.2

TABLE XXI. LTAR for LPP systems based on Soil Group and texture class

TABLE XXII. LTAR for LPP s	ystems in saprolite based on Sa	prolite Group and texture class

Saprolite Group	Saprolite Textural Class		LTAR in gpd/ft <sup>2</sup>
Ι	Sands	Sand	0.3 - 0.4
		Loamy Sand	0.25 - 0.35
II	Loams	Sandy Loam	0.2 - 0.3
		Loam	0.1 - 0.2
		Silt Loam	0.05 - 0.15

(d) The minimum required dispersal field area and trench length shall be calculated in accordance with the following:

(1) the minimum required dispersal field area shall be calculated by dividing the DDF by the LTAR; and

(2) the minimum trench length shall be calculated by dividing the required dispersal field area by a lateral spacing of five feet. The following equation shall be used to calculate the minimum line length required.

	TL	=	(DDF / LTAR) / LS
Where	TL	=	length of trench, in feet
	DDF	=	design daily flow, in gpd
	LTAR	=	in gpd/ft <sup>2</sup>

LS = five-foot line spacing

(3) When HSE is proposed to be discharged to an LPP dispersal field with no advanced pretreatment <u>as required in Rule</u> .0402(b)(1) of this Subchapter or has not been reclassified as DSE in accordance with Rule .0402(c) of this Subchapter, a licensed professional, if required in G.S. 89C, 89E, or 89F, shall calculate the adjusted LTAR in accordance with Rule .0402(b) of this Subchapter.

(e) In addition to the requirements set forth in Rule .0901(g) of this Section, LPP system design and installation shall comply with the following, unless otherwise specified in a PIA Approval:

- (1) the LPP distribution network shall be constructed of one to two-inch diameter pressure rated Schedule 40 PVC laterals placed in gravel that meets the requirements in Rule .0902(b)(4) of this Section or other approved media;
- (2) the trench width shall be one to two feet;
- (3) trenches shall be located not less than three times the trench width on center. The minimum spacing for trenches shall be five feet on center:
- (4) trenches shall include a minimum of eight inches of gravel or other approved media, either from a PIA Approval or subsurface dispersal system listed in Section .0900 of this Subchapter. The lateral shall be installed a minimum of five inches above the infiltrative surface;
- (5) laterals, manifolds and LPP fields shall comply with the following design criteria:
  - (A) the maximum lateral length shall yield no more than a 10 percent difference in orifice delivery rate between the first and last orifice along the lateral;
  - (B) no more than one-third of the total number of holes shall be less than 5/32 inches in diameter, with no orifices sized smaller than one-eighth inch in diameter in any lateral line;
  - (C) all orifices shall face upwards, except for two orifices, one-third of the way from the beginning and end of each lateral, which shall face downward; and
  - (D) maximum orifice spacing shall be as follows: Soil Group I five feet; Soil Group II six feet; Soil Group III eight feet; and Soil Group IV 10 feet;
- (6) the orifices shall be protected by the following:
  - (A) lateral sleeved within a three or four-inch perforated corrugated or smooth wall tubing meeting the requirements of Rule .0703(d) of this Subchapter; or
  - (B) orifice shields that prevent aggregate, soil, and tree roots from clogging the orifices;
- (7) the following additional design provisions shall be required for sloping sites:
  - (A) separately valved manifolds shall be required for all subfield segments where the elevation difference between the highest and lowest laterals exceeds three feet;
  - (B) the orifice spacing, orifice size or both shall be adjusted to compensate for relative elevation differences between laterals branching off a common supply manifold and to compensate for the lines at the lowest elevation receiving more effluent at the beginning and end of a dosing cycle;
  - (C) the lateral network shall be designed to achieve a 10 to 40 percent higher steady state flow rate per linear foot into the upper lines, relative to the lower lines, depending on the amount of elevation difference and the number of laterals. The steady state flow rate is based on the pipe being full;
  - (D) maximum elevation difference between the highest and lowest laterals in a field shall not exceed 10 feet unless the flow is uniformly divided using multiple pumps or split between subfield segments without requiring simultaneous adjustment of multiple pressure regulating valves in separate locations. Flow shall be uniformly divided such that the dose volumes to the subfields does not vary more than 10 percent on an area basis; and
  - (E) the Department shall approve other designs based upon the authorized designer or PE providing documentation showing equivalent hydraulic performance to this Subparagraph;
- (8) turn-ups shall be provided at the ends of each lateral, constructed of Schedule 40 PVC pipe or stronger pressure-rated pipe, and shall terminate at the ground surface and be installed in a valve box or equivalent that provides access for operation and maintenance;
- (9) the supply manifold shall be constructed of solvent-welded pressure rated Schedule 40 PVC;
- (10) the supply manifold shall be sized large enough based on the size and number of laterals served to prevent more than a 20 percent variation in pressure head between the first and last laterals due to losses within the manifold when feeding the manifold from a lower elevation;
- (11) the supply manifold shall comply with the following design criteria:
  - (A) the ratio of the supply manifold inside cross-sectional area to the sum of the inside cross-sectional areas of the laterals served shall exceed 0.7:1 as measured from where the supply line connects to the manifold;
  - (B) the reduction between the manifold and connecting laterals shall be made off the manifold using reducing tees or fittings; and
  - (C) cleanouts shall be installed at the distal ends of the supply manifold and shall be enclosed in valve boxes accessible from the ground surface;
- (12) pressure regulating valves shall be provided for pressure adjustment at the fields;
- (13) valves shall be installed in an access device, such as a valve box, and be accessible and operable from the ground surface. Valves serving contiguous subfields shall be in a common valve box;

- (14) the LPP dosing system shall comply with the following design criteria:
  - (A) the pump operating flow rate shall be based upon delivering three feet to six feet of residual pressure head at the distal end of all laterals;
  - (B) the dose volume shall be between five and 10 times the liquid capacity of the lateral pipe dosed, plus the liquid capacity of the portions of manifold and supply lines which drain between doses; and
  - (C) when pumping downhill and the supply line volume exceeds 20 percent of the calculated dose volume, special design considerations shall be followed to prevent more than 20 percent of the dose volume from draining by gravity to the dispersal field between doses; and
- (15) the trenches shall be covered to a minimum depth of four inches after settling.

(f) The authorized agent or Department may approve on a site-specific basis drip dispersal systems used in LPP trenches and other LPP designs based on documentation showing that the proposed design meets the performance requirements of this Rule.

History Note: Authority G.S. 130A-335(e) and (f); <u>S.L. 2024-49, s.4.33;</u> Eff. January 1, 2024.

#### 15A NCAC 18E .0908 DRIP DISPERSAL SYSTEMS

(a) This Rule provides for the permitting of drip dispersal systems receiving DSE. Drip dispersal systems shall comply with the provisions of this Rule and Section .1600 of this Subchapter.

- (b) Drip dispersal systems with advanced pretreatment shall comply with Rule .1204 of this Subchapter.
- (c) Drip dispersal systems <u>receiving DSE</u> shall meet the following soil and site criteria:
  - (1) A minimum of 18 inches of naturally occurring suitable soil above a LC, 13 inches of naturally occurring suitable soil above a SWC, and the minimum vertical separation to any LC shall be 12 inches. A groundwater lowering system may be used to comply with the vertical separation to a SWC when only Group I or II soils with suitable structure are present within 36 inches of the naturally occurring soil surface.
  - (2) For new fill, the soil and site shall meet the following criteria:
    - (A) Rule .0909(b) and (c) of this Section, except as otherwise specified in this Subparagraph;
    - (B) no SWC shall exist within the first 12 inches below the naturally occurring soil surface. A groundwater lowering system shall not be used to comply with the initial site requirements for a new fill system; and
    - (C) minimum vertical separation to any unsuitable soil horizon or rock shall be 18 inches and 12 inches for any SWC.
  - (3) For existing fill, the soil and site shall meet the following criteria:
    - (A) Rule .0909(d) and (e) of this Section, except as otherwise specified in this Subparagraph; and
      - (B) minimum vertical separation to any LC shall be 24 inches.
- (d) Tables XXIII and XXIV shall be used to determine the LTAR for all DSE drip dispersal systems:
  - (1) Table XXIII shall be used for systems utilizing soil. The LTAR shall be based on the most limiting, naturally occurring soil horizon within 18 inches of the naturally occurring soil surface or to a depth of 12 inches below the infiltrative surface, whichever is deeper;
  - (2) Table XXIV shall be used for systems utilizing saprolite. The LTAR shall be based on the most limiting, naturally occurring saprolite to a depth of 24 inches below the infiltrative surface;
  - (3) the LTAR for new fill systems shall not exceed 0.5 gpd/ft<sup>2</sup> for Group I, 0.3 for gpd/ft<sup>2</sup> Group II, 0.15 gpd/ft<sup>2</sup> for Group III or 0.05 gpd/ft<sup>2</sup> for Group IV soils, respectively;
  - (4) sections of blank tubing without emitters shall not count towards the minimum dripline length required; and
  - (5) the DDF shall be divided by the LTAR, determined from Table XXIII or XXIV, to determine the minimum dispersal field area required. The minimum dripline length shall be determined by dividing the required area by the maximum line spacing of two feet. The designer may recommend additional linear footage as soil and site conditions allow. The following equations shall be used to calculate the minimum dispersal field area and dripline length required:

	MA	=	DDF / LTAR
	DL	=	MA / LS
Where	MA	=	minimum dispersal field area, in ft <sup>2</sup>
	DDF	=	design daily flow, in gpd
	LTAR	=	in gpd/ft <sup>2</sup>
	DL	=	dripline length, in feet
	LS	=	two-foot line spacing

Soil Group	USDA Soil Textural Class		LTAR in gpd/ft <sup>2</sup>
т	Sanda	Sand	0.4 - 0.6
1	Sands	Loamy Sand	0.4 - 0.0
п	Course Loome	Sandy Loam	0.3 - 0.4
11	Coarse Loams	Loam	0.3 - 0.4

		Sandy Clay Loam	
		Silt Loam	
III	Fine Loams	Clay Loam	0.15 - 0.3
		Silty Clay Loam	
		Silt	
		Sandy Clay	
IV	Clays	Silty Clay	0.05 - 0.2
		Clay	

TABLE XXIV. LTAR for DSE drip dispersal systems based on S	Saprolite Group and texture class
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Saprolite Group	Saprolite Textural Class	LTAR in gpd/ft <sup>2</sup>
Ι	Sand	0.3 - 0.4
	Loamy sand	0.25 - 0.35
II	Sandy loam	0.2 - 0.3
	Loam	0.1 - 0.2
	Silt Loam	0.05 - 0.1

- (e) A special site evaluation shall be required in accordance with Rule .0510 of this Subchapter, as applicable.
- (f) Drip dispersal installation shall be in accordance with the following criteria:
  - (1) dripline shall be installed in accordance with the approved design. The design shall specify installation depth, installation equipment, blanking, drainback prevention, and any other site-specific design requirements identified by the designer;
  - (2) dripline shall be installed a minimum of one inch into naturally occurring soil, except when installed in a fill system;
  - (3) driplines shall be installed level. A maximum variance of plus or minus two inches shall be allowed within any contiguous section of dripline containing drip emitters;
  - (4) a minimum of six inches of cover shall be maintained over the dripline. The six inches of cover may be met by the addition of up to six inches, after settling, of suitable Group II or III soil over the drip field;
  - (5) drip dispersal fields shall be sloped to shed surface water;
  - (6) if cover material is required and the slope is greater than 30 percent, a slope stabilization plan shall be provided by a licensed professional if required in G.S. 89C, 89E, or 89F; and
  - (7) the drip dispersal system shall be field tested after installation in accordance with Rule .1603 of this Subchapter.

History Note: Authority G.S. 130A-335(e) and (f); <u>S.L. 2024-49, s.4.34;</u> Eff. January 1, 2024.

# 15A NCAC 18E .0909 FILL SYSTEMS

(a) Both new and existing fill systems are a system in which all or part of the dispersal field media is installed in fill material. The system includes both the basal area of dispersal field and the toe slope in all directions.

- (b) New fill systems may be installed on sites that meet the following requirements:
  - (1) a minimum of the first 18 inches below the naturally occurring soil surface consists of suitable soil with the exception that no SWC exists within the first 12 inches below the naturally occurring soil surface and a groundwater lowering system is not used to meet this requirement;
  - (2) systems shall be installed only on sites with uniform slopes less than four <u>15</u> percent;
  - (3) stormwater diversions, subsurface interceptor drains, or swales shall be required as needed upslope of the system to divert surface runoff or lateral flow from passing over or into the system; and
  - (4) the area of suitable soil shall be large enough to include the basal area of dispersal field and the toe slope in all directions.
- (c) New fill system design and installation shall be in accordance with the following criteria:
  - (1) trenches shall be installed with a minimum of 24 inches separating the infiltrative surface and any LC for gravity distribution and pressure dosed gravity distribution, except for any SWC that requires 18 inches of separation. If pressure dispersal is used, the minimum separation distance shall be 18 inches between the infiltrative surface and any LC and 12 inches to a SWC. This separation requirement may be met with the use of a groundwater lowering system only in Soil Groups I and II with suitable structure;
  - (2) fill systems with a DDF greater than 480 gpd shall use pressure dispersal systems;
  - (3) fill material soil texture shall be classified as Group I up to the top of the trenches. The final six inches of fill used to cover the system shall have a finer texture, such as Group II or III soils, for the establishment of a vegetative cover;
  - (4) minimum cover shall be six inches after settling;
  - (5) additional fill may be added to facilitate drainage and accommodate final landscaping requirements at the site necessary to stabilize the fill, shed surface water, and establish a vegetative cover. The additional fill may be provided if the infiltrative surface is less than 30 inches below the finished grade;

- (6) where fill material is added, the fill material and the existing soil shall be mixed to a depth of six inches below the interface. Vegetative cover, organic litter, and the O horizon shall be removed before the additional fill material is incorporated;
- (7) the fill system shall be constructed as an elongated berm with the long axis parallel to the ground elevation contours of the slope;
- (8) the side slope of the fill system shall not exceed a rise to run ratio of 1:4. If the first 18 inches below the naturally occurring soil surface is Group I soil, the side slope of the fill shall not exceed a rise to run ratio of 1:3;
- (9) the outside edge of the trench shall be located a minimum of five feet horizontally from the top of the side slope;
- (10) the fill system shall be shaped to shed surface water and shall be stabilized with a vegetative cover;
- (11) trench products approved under Section .1700 of this Subchapter shall be installed in accordance with PIA Approval; and
- (12) the setback requirements shall be measured from the projected toe of the slope. If this setback cannot be met, the setback requirements shall be measured five feet from the nearest edge of the trench if the following conditions are met:
  - (A) slope of the site does not exceed two percent;
  - (B) the first 18 inches of soil beneath the naturally occurring soil surface shall consist of Group I soils; and
  - (C) the lot or tract of land was recorded on or before December 31, 1989.
- (d) An existing pre-July 1, 1977 fill site that does not meet the requirements of Paragraph (b) of this Rule may be utilized for a wastewater system if the following requirements are met:
  - (1) substantiating data are provided by the lot owner indicating that the fill material was placed on the site prior to July 1, 1977;
  - (2) the fill material shall have Group I soil texture for a minimum depth of 24 inches below the existing ground surface;
  - (3) the fill material shall have no more than 10 percent by volume of fibrous organics, building rubble, or other debris, and shall not have discreet layers containing greater than 35 percent of shell fragments;
  - (4) if a minimum of 24 inches of Group I fill material is present, additional fill with soil texture classified Group I may be added to comply with the separation requirements of Subparagraph (e)(5) of this Rule;
  - (5) SWC is 18 inches or greater below the ground surface of the fill. This requirement shall be met without the use of a groundwater lowering system; and
  - (6) the area of suitable soil shall be large enough to include the basal area of dispersal field and the toe slopes in all directions.
- (e) Existing fill system design and installation shall be in accordance with Paragraph (c) of this Rule and the following criteria:
  - (1) the DDF shall not exceed 480 gpd;
  - (2) pressure dispersal shall be used. LPP systems shall meet the requirements of Rule .0907(d) and (e) of this Section. Drip dispersal systems shall meet the requirements of Rule .0908(d) and (f) of this Section;
  - (3) the LTAR shall not exceed 0.5 gpd/ft<sup>2</sup> for pressure dispersal systems;
  - (4) existing fill sites with 48 inches of Group I soils may use conventional trenches with a maximum LTAR of 1.0 gpd/ft<sup>2</sup>
     in lieu of a pressure dispersal system;
  - (5) the minimum vertical separation to any LC shall be 24 inches for pressure dispersal systems and 48 inches for conventional systems. This vertical separation requirement may be met by adding additional Group I soil, but shall not be met with the use of a groundwater lowering system;
  - (6) where additional Group I fill is to be added, the side slope of the fill shall not exceed a side slope ratio of 1:3; and
  - (7) trench products approved under Section .1700 of this Subchapter shall be installed in accordance with their PIA Approval.
- (f) The LTAR for new and existing fill systems shall be determined in accordance with Rule .0901(c) of this Section and the following:
  - (1) the LTAR shall be based on the most limiting, naturally occurring soil horizon within 18 inches of the ground surface or to a depth 12 inches below the infiltrative surface, whichever is deeper;
  - (2) the lowest LTAR for the applicable Soil Group shall be used for systems installed in accordance with this Rule; and
  - (3) for sites with a minimum of 18 inches of Group I soils below the naturally occurring soil surface or to a depth of 12 inches below the infiltrative surface, whichever is deeper, the LTAR shall not exceed 1.0 gpd/ft<sup>2</sup> for gravity or pressure dosed gravity distribution or 0.5 gpd/ft<sup>2</sup> for pressure dispersal systems.

(g) The authorized agent or Department may approve other fill system designs on a site-specific basis in accordance with a PIA Approval or Rule .0509(c) of this Subchapter.

History Note: Authority G.S. 130A-335(e) and (f); <u>S.L. 2024-49, s.4.35;</u> Eff. January 1, 2024.

# SECTION .1100 – SYSTEM DOSING AND CONTROLS

### 15A NCAC 18E .1101 GENERAL DOSING SYSTEM REQUIREMENTS

(a) Dosing systems with a single pump or siphon shall be required to be used to deliver effluent into laterals when:

- (1) gravity distribution cannot be achieved between the septic tank and dispersal field;
- (2) the total lateral length exceeds 750 linear feet in a single system; or

(3) a pressure dosed gravity distribution or pressure dispersal system is used.

(b) Dosing systems with multiple alternating or sequencing pumps or siphons shall be used to discharge to separate dispersal fields when:

(1) DDF from a single system exceeds 3,000 gpd; or

(2) the total line length exceeds 2,000 linear feet in a single trench system or 5,000 linear feet in a drip dispersal system.
 (c) If alternating pumps or siphons are not required in accordance with Paragraph (b) of this Rule, but used, then the alternating pumps or siphons may discharge to a single dispersal field.

(d) The dose volume to a dispersal field shall be calculated as follows:

- (1) 66 to 75 percent of the volume of the installed linear lateral footage for pressure dosed gravity distribution systems;
- (2) 66 to 75 percent of the volume of the installed linear lateral footage for LDP systems and trench products with a PIA approval based on lateral capacity equivalent to the capacity of a four-inch corrugated pipe;
- (3) LPP systems in accordance with Rule .0907(e)(14)(B) of this Subchapter; and
- (4) drip dispersal systems in accordance with Rule .1602(f)(3) of this Subchapter.

(e) The pump operating flow rate from a dosing system shall be designed to achieve scour velocity in the supply line and to distribute effluent in accordance with the dispersal field design.

(f) The pump operating flow rate or average pump run time shall be within 25 percent of the initial measurements collected during the final inspection.

(g) All dosing systems shall be tested using water prior to issuance of an OP. OP or the ATO. The test shall may be conducted by the installer. installer, The installer shall give the LHD, LSS, authorized designer, AOWE, and or PE, as applicable, the option to witness the test. witnessed by the LHD, A PE and AOWE shall be authorized to witness the dosing tests for systems they have designed and for LHD permits upon a signed acceptance of responsibility for the verification of the dosing system. The dosing test shall and include a demonstration and documentation of the following:

- (1) pump or siphon operating flow rate and dose volume delivered;
  - (2) float control levels;
  - (3) high-water alarm, including sound;
  - (4) operating pressure head, if applicable; and
  - (5) delivery of water to the dispersal field.

(h) Documentation of the dosing system test results shall be provided to the LHD, PE, or AOWE for attachment to the permit or NOI, as applicable.

History Note: Authority G.S. 130A-335(e), (f), and (f); <u>S.L. 2023-77, s.18;</u> Eff. January 1, 2024.

#### SECTION .1200 - ADVANCED PRETREATMENT SYSTEMS STANDARDS, SITING, AND SIZING CRITERIA

# 15A NCAC 18E .1202 SITING AND SIZING CRITERIA FOR ADVANCED PRETREATMENT SYSTEMS WITH A DESIGN DAILY FLOW LESS THAN OR EQUAL TO 1,500 GALLONS/DAY

(a) Wastewater systems utilizing advanced pretreatment with a DDF less than or equal to 1,500 gpd may only use one of the following modifications to system siting and sizing criteria, unless otherwise identified in this Rule:

- (1) reduction in depth to LC or vertical separation to LC in accordance with Paragraph (b) of this Rule;
- (2) LTAR increase in accordance with Paragraph (c) of this Rule; or
- (3) setback reductions in accordance with Paragraph (d) of this Rule.

(b) The minimum required vertical separation to a LC in natural soil may be reduced with the use of advanced pretreatment in accordance with Table XXVI. Table XXVII provides the minimum depths and vertical separation for new and existing fill. A special site evaluation shall be submitted and approved in accordance with Rule .0510 of this Subchapter when a reduction in vertical separation to a LC is proposed in accordance with this Rule.

Table XXVI. Minimum vertical separation to LC based on effluent standards for wastewater systems with a DDF less than or equal to

1,500 gpd

Ν	Minimum vertical separation in inches from infiltrative surface to LC					
Soil Group	Distribution	Effluent Standard**				
	Method	DSE*	NSF/ANSI 40	TS-I	TS-II	
Ι	Gravity	18	12	12	12	
	LPP	12	12	9	6	
	Drip	12	12	9	6	
II-IV	Gravity	12	12	9	9	
	LPP	12	12	9	6	
	Drip	12	12	9	6	

\*For comparison

\*\*12-inch vertical separation shall always be maintained to rock or tidal water

**Table XXVII.** Minimum depth to LC and vertical separation to SWC in new or existing fill based on effluent standards for wastewater systems with a DDF less than or equal to 1,500 gpd for new fill and less than or equal to 480 gpd for existing fill

Minimum dep	oth in inches from <b>n</b>	aturally occurr	ing soil surface of	r existing fill su	rface to LC	
Type of Fill	Distribution of Fill Method			Effluent Standard		
		DSE**	NSF/ANSI 40	TS-I	TS-II	
New Fill	Gravity	18 to LC 12 to SWC	18 to LC 12 to SWC	14 to LC 12 to SWC	14 to LC 12 to SWC	
	LPP	18 to LC 12 to SWC	18 to LC 12 to SWC	12	12	
	Drip	18 to LC 12 to SWC	18 to LC 12 to SWC	12	12	
Existing Fill	Gravity LPP	24 of Group I Fill or Soil to L 18 of Group I Fill or Soil to SV				
	Drip		18 01 010up 1 F1		C	
Mir	nimum vertical sepa	aration in inche	s from infiltrative	e surface to LC	*	
Type of Fill	Distribution Method	Effluent Standard				
		DSE**	NSF/ANSI 40	TS-I	TS-II	
New Fill	Gravity	24 to LC 18 to SWC	18 to LC 18 to SWC	18 to LC 14 to SWC	18 to LC 14 to SWC	
	LPP	18 to LC 12 to SWC	18 to LC 12 to SWC	12 to LC 9 to SWC	9 to LC 6 to SWC	
1	D '	10 . 10	10 . 10	10 . 10	0.1.1.0	

		18 to SWC	18 to SWC	14 to SWC	14 to SWC
	LPP	18 to LC	18 to LC	12 to LC	9 to LC
		12 to SWC	12 to SWC	9 to SWC	6 to SWC
	Drip	18 to LC	18 to LC	12 to LC	9 to LC
		12 to SWC	12 to SWC	9 to SWC	6 to SWC
Existing Fill	Gravity	48	36	24	24
	LPP	24	18	12	12 to LC
					9 to SWC
	Drip	24	18	12	12 to LC
	_				9 to SWC

\*Minimum depth after adjustment for slope correction

\*\*For comparison

(c) The LTAR shall be based on the effluent standard and dispersal field type proposed in accordance with the following:

- (1) The LTAR may be increased by the following factors when compared to the rate assigned by the authorized agent for a new system using DSE:
  - (A) up to 1.33 for NSF/ANSI 40 effluent standards in soils which are Group I or II with suitable structure;
  - (B) up to 2.0 for TS-I or TS-II effluent standards when pressure dispersal is utilized; or
  - (C) up to 2.5 for TS-II effluent standards when all the following conditions are met: minimum of 36 inches of Group I soils from the naturally occurring soil surface; minimum depth to a SWC below the naturally occurring soil surface is 24 inches; space shall be available for an equivalently sized dispersal field repair area; and pressure dispersal shall be utilized.
- (2) A special site evaluation, if required in accordance with Rule .0510 of this Subchapter, shall be submitted and approved.
- (3) The LTAR for an aerobic drip system shall be determined in accordance with Rule .1204 of this Section.
- (4) Trench dispersal products approved for a specific dispersal field reduction in area or trench length when receiving DSE in accordance with this Subchapter or a PIA Approval shall not be reduced by more than 50 percent when any LTAR adjustments are taken in accordance with this Rule.
- (5) When using pressure dispersal systems, the proposed LTAR increases in Subparagraph (c)(1) may be used concurrently with the reduced setbacks for TS-II Systems in Table XXVIII.
- (6) The DDF shall not be increased by the addition of advanced pretreatment to an existing wastewater system by more than 33 and one-third percent on a site without repair area or by more than 50 percent on a site with 100 percent repair area.
- (d) Advanced pretreatment systems shall meet the following setback requirements:
  - (1) minimum setback requirements of Section .0600 of this Subchapter shall be met, except as shown in Table XXVIII; and
  - (2) when any other siting or sizing modifications are applied, such as reduced depth to LC, vertical separation, or increased LTAR, for a TS-I or TS-II system in accordance with Paragraphs (b) and (c) of this Rule, no setback reductions shall

be taken except those to artificial drainage systems described in Table XXVII, unless otherwise specified in this Section.

Site Features	Setback in feet according to Effluen Standard**		ffluent	
	DSE*	NSF/ANSI 40	TS-I	TS-II
Surface waters classified WS-I, from ordinary high-water mark	100	70	70	50
Waters classified SA, from mean high-water mark	100	70	70	50
Any Class I or Class II reservoir, from normal water level	100	70	70	50
Any other stream, non-water supply spring, or other surface water, from the ordinary high-water mark	50	35	35	25
Tidal influenced waters, such as marshes and coastal water, from mean high-water mark	50	35	35	25
Lake or pond, from normal water level	50	35	35	25
Groundwater lowering system, as measured on the ground surface from the edge of the feature	25	25	20	15
Downslope interceptor drains and surface water diversions with a vertical cut of more than two feet, as measured on the ground surface from the edge of the feature	15	15	10	10
Upslope and side slope interceptor drains and surface water diversions with a vertical cut of more than two feet, as measured on the ground surface from the edge of the feature	10	10	7	5
A stormwater collection system as defined in 15A NCAC 02H .1002(48), excluding gutter drains that connect to a stormwater collection system, with a vertical cut of more than two feet as measured from the center of the collection system	10	10	7	5
Permanent stormwater retention basin, from normal water level	50	50	35	25
Any other dispersal field, except designated dispersal field repair area for project site	20	20	10	5

Table XXVIII: Setbacks for wastewater s	vstems meeting NSF/ANSI 4	0. TS-I. or TS-II effluent standards
	jetenne meeting i ter i ter ter te	

\*For comparison

(1)

(2)

\*\*May require a variance from DEQ based on local buffer rules.

(e) Sandy clay loam saprolite may be used with advanced pretreatment meeting NSF/ANSI 40, TS-I, or TS-II effluent standards.

History Note: Authority G.S. 130A-334; 130A-335; 130A-342; 130A-343; <u>S.L. 2024-49, s.4.36;</u> Eff. January 1, 2024.

# 15A NCAC 18E .1203 SITING AND SIZING CRITERIA FOR ADVANCED PRETREATMENT SYSTEMS WITH A DESIGN DAILY FLOW GREATER THAN 1,500 GALLONS/DAY AND LESS THAN OR EQUAL TO 3,000 GALLONS/DAY

(a) Wastewater systems utilizing advanced pretreatment with a DDF greater than 1,500 gpd and less than or equal to 3,000 gpd may utilize the system siting and sizing in this Rule.

(b) The LTAR shall be based on the effluent standard and dispersal field type proposed in accordance with the following:

- The LTAR may be increased by the following factors when compared to the rate assigned by the authorized agent for a new system using DSE:
  - (A) up to 2.0 for TS-I or TS-II effluent standards; or
  - (B) up to 2.5 for TS-II effluent standards when there is a minimum of 48 inches of Group I soils from the naturally occurring soil surface and a minimum of 30 inches to a SWC below the naturally occurring soil surface.

The LTAR for an aerobic drip system shall be determined in accordance with Rule .1204 of this Section.

(c) When the LTAR for a system is proposed to be increased in accordance with Paragraph (b) of this Rule, the following conditions shall be met:

- (1) a special site evaluation required in accordance with Rule .0510 of this Subchapter shall be submitted and approved;
- (2) pressure dispersal shall be utilized;
- (3) space shall be available for an equivalently sized dispersal field repair area; and

(4) 25-foot setback shall be maintained to all property lines unless a site-specific nitrogen migration analysis for a TS-I system indicates that the nitrate-nitrogen concentration at the property line will not exceed 10 mg/L or a TS-II system is used.

(d) Trench dispersal products approved for a specific dispersal field reduction in area or trench length when receiving DSE in accordance with this Subchapter or a PIA Approval shall not be reduced by more than 50 percent as a result of increased LTAR in accordance with this Rule.

(e) The DDF shall not be increased by the addition of advanced pretreatment to an existing wastewater system.

(f) Sandy clay loam saprolite may be used with advanced pretreatment meeting NSF/ANSI 40, TS-I, or TS-II effluent standards.

(f)(g) Wastewater systems utilizing advanced pretreatment with a DDF greater than 3,000 gpd may propose LTAR adjustments in accordance with Paragraphs (a) through (c) of this Rule. Sandy clay loam saprolite may be used with advanced pretreatment with a DDF greater than 3,000 gpd. The Department shall review and approve the proposed LTAR adjustments in accordance with Rule .0302(e) of this Subchapter. Documentation shall also be provided that the proposed system meets the requirements of Rule .0510(e) of this Subchapter.

History Note: Authority G.S. 130A-334; 130A-335; 130A-342; 130A-343; <u>S.L. 2024-49, s.4.37;</u> Eff. January 1, 2024.

# 15A NCAC 18E .1205 ADVANCED PRETREATMENT SAND LINED TRENCH SYSTEMS

(a) Sand lined trench systems with a DDF less than or equal to 1,500 gpd receiving TS-I or TS-II effluent shall meet the requirements of this Rule.

(b) The site meets the criteria in Rule .0906(c) of this Subchapter and the receiving permeable horizon may be deeper than 60 inches below the natural grade.

(c) If a groundwater lowering system is used to comply with the vertical separation to a SWC, the following conditions shall apply:

- (1) the site shall comply with the requirements of Rule .0906(d) of this Subchapter; and
- (2) the vertical separation requirement to a SWC shall be reduced to nine inches with pressure dosed gravity distribution or six inches with pressure dispersal.

(d) Table XXXI shall be used to determine the LTAR for a sand-lined trench system and shall be based on the most limiting, naturally occurring soils overlying the permeable receiving layer. An equivalent trench width of three feet shall be used to determine trench length in accordance with Rule .0901(d) of this Subchapter. The LTAR shall be one of the following:

- (1) the rate set forth in Table XXXI; or
- (2) 20 percent of the in-situ Ksat of the receiving permeable horizon, whichever is less.

**TABLE XXXI.** LTAR for advanced pretreatment sand lined systems based on texture of the most hydraulically limiting overlying soil horizon

Soil Group	Texture of Most Hydraulically Limiting Overlying Soil Horizon	LTAR in gpd/ft <sup>2*</sup>
Ι	Sand	0.9 - 1.4
II	Coarse Loams	0.7 - 1.0
III	Fine Loams	0.4 - 0.8
IV	Clays	0.2 - 0.4

\* There shall be no reduction in trench length compared to a conventional gravel trench when Accepted or Innovative gravelless trench product is used. Trench length for trench dispersal products approved with a specific dispersal field reduction in area or trench length when receiving DSE effluent in accordance with this Subchapter or a PIA approval shall be calculated in accordance with this Subchapter or the PIA approval.

(e) A Special Site Evaluation in accordance with Rule .0510 of this Subchapter shall be required for the following conditions to field verify the LTAR:

- (1) when the texture of the receiving permeable horizon is sandy loam or loam, and the system DDF is greater than 600 gpd; or
- (2) when the texture of the receiving permeable horizon is silt loam.
- (f) Setbacks in accordance with Table XXVIII of Rule .1202(d) of this Section shall be applied to sand lined trench systems.

(g) Sand lined trench system installation shall be in accordance with Rule .0906(h) of this Subchapter.

History Note: Authority G.S. 130A-334; 130A-335; 130A-342; 130A-343; <u>S.L. 2024-49, s.4.38;</u> Eff. January 1, 2024.

## SECTION .1300 – OPERATION AND MAINTENANCE

#### 15A NCAC 18E .1301 OPERATION AND MAINTENANCE OF WASTEWATER SYSTEMS

(a) Wastewater systems shall be operated and maintained in accordance with the conditions of the OP, PIA Approval, and the Rules of this Section, including the manufacturer's operation and maintenance instructions, as applicable. Dispersal field repair areas shall be maintained in accordance with the Rules of this Subchapter.

(b) System management in accordance with Table XXXII shall be required for all systems installed or repaired after July 1, 1992. System management in accordance with Table XXXII shall also be required for all Type V and VI systems installed on or before July 1, 1992.

TABLE XXXII. Management responsibilities based on wastewater system classification type and description					
System Classification Type and Description	LHD Compliance Inspection Frequency	Management Entity	Management Entity Minimum Maintenance Inspection Frequency		
Ia – Privy or vault privy	N/A	Owner	N/A		
Ib – Chemical toilet	N/A	Owner	N/A		
Ic – Incinerating toilet	N/A	Owner	N/A		
Id – Composing toilet system	N/A	Owner	N/A		
Ie – Other toilet system	N/A	Owner	N/A		
IIa – Conventional system for a single family or 480 gpd or less with 750 linear feet of trench or less	N/A	Owner	N/A		
IIb – Accepted wastewater gravity system	N/A	Owner	N/A		
IIIa Conventional wastewater system greater than 480 gpd excluding single family residences	N/A	Owner	N/A		
IIIb – Wastewater system with a single	5 years	Owner or	N/A		
pump or siphon	Ň/A	Certified Operator	5 years		
IIIc – Gravity fill system	N/A	Owner	N/A		
IIId – Alternating dual fields with gravity distribution	N/A	Owner	N/A		
IIIe – PPBPS gravity system	N/A	Owner	N/A		
IIIf – LDP gravity system	N/A	Owner	N/A		
IIIg – Other non-conventional systems	N/A	Owner	N/A		
IIIh – Gravity groundwater lowering system	5 years	Owner	N/A		
IVa – LPP distribution	3 years	Private Certified Operator or Public Management Entity with a Certified Operator	2/year		
IVb – System with more than one pump or siphon	3 years	Private Certified Operator or Public Management Entity with a Certified Operator	2/year		
IVc – Off-site system serving two or more facilities with any components under common or joint control	5 years	Private Certified Operator or Public Management Entity with a Certified Operator	1/year		
IVd –Alternating dual fields with pressure dosed gravity distribution including off-site systems	3 years	Private Certified Operator or Public Management Entity with a Certified Operator	1/year		
Va – Advanced pretreatment meeting NSF/ANSI 40, TS-I, or TS-II, approved under Section .1700 of this Subchapter, $DDF \le 3,000$ gpd	1/year	Private Certified Operator or Public Management Entity with a Certified Operator	≤ 1,500 gpd - 2/year* > 1,500 gpd and ≤ 3,000 gpd - 4/year		
Vb – DSE wastewater systems > 3,000 gpd with dispersal field > 1,500 gpd	1/year	Private Certified Operator or Public Management Entity with a Certified Operator	> 3,000 and ≤ 10,000 gpd - monthly > 10,000 gpd flow - weekly		

TABLE XXXII. Management responsibilities based on wastewater system classification type and description

		1	
Vc - RWTS, approved under Section	1/year	Private Certified Operator	$\leq$ 1,500 gpd - 2year*
.1500 of this Subchapter, meeting		or Public Management	
NSF/ANSI 40, DDF $\leq$ 1,500 gpd		Entity with a Certified	
		Operator	
Vd - Anaerobic drip dispersal systems	1/year	Private Certified Operator	≤ 1,500 gpd - 2/year*
		or Public Management	$> 1,500$ gpd and $\le 3,000$ gpd - 4/year
		Entity with a Certified	$> 3,000$ gpd and $\leq 10,000$ gpd -
		Operator	12/year
		1	> 10,000 gpd – 1/week
Ve - Flow equalization	$\leq$ 1,500 gpd – once	Private Certified Operator	Based on equalized flow
1	every three years	or Public Management	$\leq 1,500 \text{ gpd} - 2/\text{year}$
	> 1,500  gpd - 1/year	Entity with a Certified	$> 1,500 \text{ and } \le 3,000 \text{ gpd} - 4/\text{year}$
		Operator	$> 3,000 \text{ gpd} \text{ and } \leq 10,000 \text{ gpd} -$
			12/year
			>10,000 gpd – 1/week
Vf – Sand lined trench system with no	1/year	Private Certified Operator	1/year
advanced pretreatment or drip		or Public Management	
dispersal		Entity with a Certified	
I		Operator	
Vg - Wastewater system with pump	1/year		2/year with one visit during the wet
groundwater lowering systems	J	or Public Management	season
88,		Entity with a Certified	
		Operator	
		of mon	
Vh - IPWW designed by a PE and	1/year	Private Certified Operator	≤ 1,500 gpd - 2/year*
reviewed by the Department and		or Public Management	$> 1,500$ gpd and $\le 3,000$ gpd - 4/year
determined to be IPWW		Entity with a Certified	$> 3,000$ gpd and $\leq 10,000$ gpd -
		Operator	12/year
		1	> 10,000 gpd – 1/week
Vi – Permanent pump and haul	1/year	Private Certified Operator	
1 1	2	1	
VIa – Advanced pretreatment > 3,000	6 months	Private Certified Operator	Media filters
gpd meeting NSF/ANSI 40, TS-I, or	•	or Public Management	$>$ 3,000 gpd and $\le$ 10,000 gpd - 12/year
TS-II		Entity with a Certified	>10,000  gpd - 1/week
		Operator	
		operator	All other advanced pretreatment
			$>$ 3,000 gpd and $\leq$ 10,000 gpd - 12/year
			$> 10,000$ gpd and $\le 10,000$ gpd $- 12/year$ $> 10,000$ and $\le 25,000$ gpd $- 2/week$
			$> 25,000 \text{ and } \le 25,000 \text{ gpd} - 2/\text{week}$
			> 50,000 gpd - 5/week
VIb – Any system using RCW	6 months	Private Certified Operator	
	0 monuio	or Public Management	> 3,000  gpd = 12/year > 3,000 and $\le 10,000 \text{ gpd} - 1/\text{week}$
		Entity with a Certified	$> 10,000 \text{ and } \le 10,000 \text{ gpd} - 17 \text{ week}$
		Operator	$> 25,000 \text{ and } \le 23,000 \text{ gpd} - 2/\text{week}$ $> 25,000 \text{ and } \le 50,000 \text{ gpd} - 3/\text{week}$
		operator	> 50,000 gpd - 5/week
	1 11 1		- 50,000 gpu - 5/week

\*Quarterly Management Entity inspections shall be required for the first year. The quarterly inspections may be reduced to twice a year if the wastewater system is in compliance with all OP conditions after the first year.

(c) Wastewater systems with multiple components shall be classified by their highest or most complex system classification type in accordance with Table XXXII to determine LHD and Management Entity responsibilities.

(d) The Department shall classify wastewater systems not identified in Table XXXII after consultation with the Water Pollution Control Systems Operators Certification Commission.

(e) The site for the wastewater system shall be accessible for monitoring, maintenance, inspection, and repair.

(f) The system shall be maintained to comply with the effluent standards specified in Table XXV of Rule .1201(a) or Rule .1002 of this Subchapter and the OP, as applicable. Influent and effluent sampling may be required for food preparation or processing facilities, IPWW, and other systems as specified in the PIA Approval or OP.

(g) The owner may submit a written request to the LHD and Department to reduce the wastewater system effluent sampling frequency, effluent sampling constituents, or Management Entity inspection frequency. The written request shall include documentation showing that the wastewater system is compliant with its OP and Rule .1302(f) of this Section.

(h) The replacement of a specific component, except tanks and dispersal media, by an identical replacement component, including pipes, blowers, pumps, disinfection components, effluent filters, and control panels and appurtenances, shall be considered maintenance.

When the replacement is performed as maintenance by the Management Entity, this activity shall be reported to the owner and LHD within 30 days of when the activity occurs.

(i) All residuals shall be removed as specified in the OP, the RWTS or PIA Approval, Rule .1303 of this Section, or as otherwise determined to be needed by the Management Entity. Residuals from the wastewater system shall be transported and disposed of in accordance with G.S. 130A, Article 9, and 15A NCAC 13B.

History Note: Authority G.S. 130A-335(e) and (f); S.L. 2015-147, s.2; <u>S.L. 2024-49, s.4.39;</u> Eff. January 1, 2024.

# 15A NCAC 18E .1303 OWNER RESPONSIBILITIES FOR WASTEWATER SYSTEM OPERATION AND MAINTENANCE

(a) Any person owning or controlling the property upon which a wastewater system is installed shall be responsible for the following items regarding the operation and maintenance of the system:

- (1) the wastewater system shall be operated and maintained to protect North Carolina ground and surface water quality standards and to prevent the following conditions:
  - (A) discharge of sewage or effluent to the surface of the ground, surface waters, or into groundwater at any time;
  - (B) back-up of sewage or effluent into the facility, building drains, collection system, freeboard volume of the tanks, or distribution system; or
  - (C) effluent within three inches of finished grade over one or more trenches based on two or more observations made not less than 24 hours apart, and greater than 24 hours after a rainfall event;
- (2) the system shall be considered to be malfunctioning when one or more of the conditions of Subparagraph (a)(1) of this Rule occur or if it is necessary to remove the contents of the tank(s) at a frequency greater than once per month in order to prevent one or more of the conditions of Subparagraph (a)(1) of the Rule. The owner shall contact the LHD when the wastewater system is <u>malfunctioning</u>. <u>malfunctioning</u> and <u>implement remedies as directed by the LHD in accordance with Rule .1306 of this Section</u>. If the system was permitted under an EOP or AOWE permit, the owner shall <u>also</u> contact the PE or AOWE when the wastewater system is malfunctioning;
- (3) wastewater systems shall be inspected, and the entire contents of all septic tank compartments shall be removed whenever the depth of both the scum and sludge is found to be more than one-third of the liquid depth in any compartment. The effluent filter shall be rinsed to remove accumulated solids that can cause the wastewater to back up into the facility or clog the system, or replaced as needed;
- (4) residuals from the wastewater system shall be transported and disposed of in accordance with G.S. 130A, Article 9, and 15A NCAC 13B;
- (5) grease traps and grease tanks shall be pumped as needed to prevent discharge of FOG from the trap or tank to the next treatment component, but no less than yearly. Grease traps and grease tanks shall be maintained in accordance with Rule .0803(h) of this Subchapter and the owner shall maintain a contract with a septage management firm. All pumping records shall be maintained on-site;
- (6) site-specific vegetation shall be established and maintained over the wastewater system and repair area to stabilize slope and control erosion;
- (7) activities that result in soil disturbance or soil compaction shall not occur over the initial and repair dispersal field area;
- (8) maintaining the wastewater system in accordance with Rule .1301(a) of this Section; and
- (9) turning the effluent flow diversion valve for alternating dual dispersal fields once a year or as specified by the PE, AOWE, or authorized designer.

(b) A contract for operation and maintenance of a wastewater system required to be maintained by a Management Entity, as specified in Table XXXII of Rule .1301(b) of this Section, shall be in effect for as long as the system is in use. A contract shall be executed between the system owner and a Management Entity prior to the issuance of an OP, unless the system owner and Management Entity are the same. The contract shall include:

- (1) specific requirements for operation, maintenance, and associated reporting;
- (2) responsibilities of the owner;
- (3) responsibilities of the Management Entity;
- (4) provisions for notification to the LHD by the owner and Management Entity upon termination of the contract; and
- (5) other requirements for the continued performance of the system, as determined by the Management Entity, LHD, and Department, as applicable.

History Note: Authority G.S. 130A-335(e) and (f); <u>S.L. 2023-77, s.19;</u> Eff. January 1, 2024.

### 15A NCAC 18E .1305 LOCAL HEALTH DEPARTMENT RESPONSIBILITIES FOR WASTEWATER SYSTEM OPERATION AND MAINTENANCE

(a) No IP, CA, or OP shall be issued for Type IV, V, or VI systems, unless a Management Entity of the type specified in Table XXXII in Rule .1301(b) of this Section is authorized and operational to carry out operation and maintenance requirements for the wastewater system as set forth in these Rules and the OP.

(b) An LHD may be the Management Entity only for systems classified Type IV, Va, Vb, Vc, Vd, Ve, Vf, and Vg and only when authorized by the local board of health.

(c) An authorized agent shall review the performance and inspection reports submitted in accordance with Rule .1304(c) of this Section and perform an on-site compliance inspection of the systems as required in Table XXXII in Rule .1301(b) of this Section. More frequent inspections may be performed by an authorized agent if requested by the system owner or the Management Entity, or specified in the PIA approval or OP.

(d) The LHD may provide the owner with the option for a private Management Entity, who is not the owner, to perform the on-site compliance inspection for Type IIIb and IIIh systems in accordance with Table XXXII in Rule .1301(b) of this Section instead of the LHD. The Management Entity shall provide to the owner and LHD a written compliance inspection report every five years. The report shall document that the wastewater system is compliant with this Subchapter, the performance standards in the OP or ATO, and conditions in the OP or the ATO.

(e) The authorized agent shall issue a written notice of non-compliance to the owner when the wastewater system is not malfunctioning in accordance with Rule .1303(a)(2) of this Section, but non-compliant with this Subchapter, the performance standards in the OP or ATO, or conditions in the <u>OP</u>, <del>OP</del> or the ATO.

(f) The LHD shall investigate malfunctions in accordance with Rule .1306 of this Section.

History Note: Authority G.S. 130A-335(e) and (f); <u>S.L. 2024-49, s.4.40;</u> Eff. January 1, 2024.

#### 15A NCAC 18E .1306 SYSTEM MALFUNCTION AND REPAIR

(a) This Rule identifies the responsibilities of the LHD and the owner when a system is malfunctioning malfunctioning, totally or partially destroyed, or otherwise determined to require repair.

(b) The LHD or Department shall issue a written NOV to the wastewater system owner in accordance with Rule .0302(c) of this Subchapter.

(c) The wastewater system shall be repaired within 30 days of the date on the NOV issued by the Department or LHD unless the NOV specifies a different time frame for the repair based on site-specific factors, such as the severity of the repair, wastewater backing up into a restaurant or discharging into SA waters, or adverse weather that delays construction of the repair. The following steps shall be followed to remedy a malfunctioning wastewater system:

- (1) The owner shall apply for a repair in accordance with Section .0200 of this Subchapter, unless only maintenance is required to bring the wastewater system into compliance.
- (2) After investigating the malfunction, the Department or LHD shall require that the <u>The</u> wastewater system be repaired to correct the malfunction and eliminate any public health hazard. The wastewater system shall be repaired so that it meets G.S. 130A, Article 11 and this Subchapter. The owner of the system may request, on a form provided by the Department, that the authorized agent, AOWE, or PE When it is not possible to bring the wastewater system into compliance with G.S. 130A, Article 11 and this Subchapter, the authorized agent shall use their best professional judgement judgement, based on education and experience, to require <u>develop</u> a repair that should enable the wastewater system to function in a manner that complies with Rule .1303(a)(1) of this Section. The <u>LHD LHD, AOWE, or PE shall document on the form the aspects of the rules that are being altered with the use of best professional judgement to repair the wastewater system. The owner of the wastewater system shall be liable for any damages caused by a system repaired in this manner and shall agree in writing to all terms and conditions set forth by the LHD, AOWE, or PE that developed the repair, including any operation and maintenance requirements. shall document that the repair uses best professional judgement on the CA and OP. This written agreement shall be attached to the CA, OP, NOI, or ATO, as applicable. Best professional judgement shall not be used when:</u>
  - (A) the IP, CA, NOI, or ATO indicates the repair area and system type. This does not preclude the owner from applying for a different wastewater system than the one specified on the permit as a repair;
  - (B)(A) there are reductions in setbacks to drinking water wells less than what is required in Rule .0601 of this Subchapter;
  - (C)(B) there are reductions in setbacks to surface water bodies greater than 50 percent of the allowed setbacks as indicated in Rule .0601 of this Subchapter; or
  - (D)(C) there is no reasonable expectation that the repaired wastewater system will function to eliminate public health <u>hazards.</u>
- (3) When necessary to protect the public health, the Department or LHD shall require the owner of a malfunctioning system to pump and haul sewage to an approved wastewater system during the time needed to repair the wastewater system. This requirement shall be included in the NOV issued to the owner.

(d) If no repair options are available for the wastewater system in accordance with Paragraph (c), the LHD may issue a CA and OP for a permanent pump and haul system. The applicant shall submit an application to the LHD for the permanent pump and haul system. The application and permanent pump and haul system shall meet the following conditions:

The owner shall provide the following information as part of the application:

(1)

- (A) a report that the system cannot be repaired by connection to a system approved under this Section or a system approved under G.S. 143, Article 21;
- (B) a contract with a septage management firm permitted in accordance with G.S. 130A-291.1 to pump and haul the sewage;

- (C) documentation that the wastewater system has been approved under this Subchapter or in accordance with 15A NCAC 02H or 15A NCAC 02T to accept sewage; and
- (D) documentation from the facility receiving the sewage confirming that the facility has the capacity for the additional sewage and agrees to accept it.
- (2) The LHD shall design the pump and haul system based on the following criteria:
  - (A) tankage with a minimum of five days storage capacity and two days emergency storage capacity;
  - (B) high-water alarm set to go off with two days of emergency storage capacity left in the tankage; and
  - (C) telemetry unit that contacts the septage management firm.
- (3) The owner of a non-residential facility may request a reduction in the five day storage requirement, if the owner can document the ability to have the tanks pumped out with only 24 hours' notice. The total tank capacity shall never be less than the minimum required septic tank and pump tank capacity required by Section .0800 of this Subchapter.
- (4) Tanks shall be approved by the LHD for permanent pump and haul if shown to be structurally sound, watertight, and of a capacity needed based on the DDF and projected pumping frequency. Existing tanks may be used for permanent pump and haul if the tanks meet the requirements in this Subparagraph.
- (5) Prior to issuing the OP, the LHD shall receive from the owner a contract with a Management Entity for inspection and maintenance of the system.
- (6) A non-transferrable OP, valid for a period of five years, shall be issued to the pump and haul system owner.

(e) A malfunctioning wastewater system that has been disconnected from the facility for any reason shall be repaired prior to reuse. (f) If the dispersal field in a malfunctioning wastewater system is found to be nonrepairable, the dispersal field shall not be used. The

(i) If the dispersal field in a manufactioning wastewater system is round to be nonceptulate, the dispersal field shall not be dised. The system owner shall be required to abandon the system to protect the public health and safety as specified in Rule .1307 of this Section.
 (g) For facilities with a malfunctioning wastewater system installed prior to July 1, 1977, the authorized agent agent, AOWE, or PE shall may use their best professional judgement, based on education and experience, judgement to repair the system.

(h) For facilities with a wastewater disposal method installed prior to July 1, 1977, which has been in continual use and acts as the sole source of wastewater disposal, the authorized agent agent, AOWE, or PE shall may use their best professional judgement, based on education and experience, judgement to repair the wastewater disposal method.

(i) Legal remedies may be pursued, in accordance with G.S. 130A, Article 1, Part 2, after an authorized agent has observed and documented one or more malfunctioning conditions and issued an NOV.

History Note: Authority G.S. 130A-291.1; 130A-291.2; 130A-335(e) and (f); <u>S.L. 2023-77, s.20; S.L. 2024-49, s.4.41;</u> Eff. January 1, 2024.

### SECTION .1400 – APPROVAL OF TANKS, RISERS, EFFLUENT FILTERS, AND PIPE PENETRATION BOOTS

#### 15A NCAC 18E .1401 PLANS FOR PREFABRICATED TANKS

(a) All tanks proposed for use in a wastewater system described in this Subchapter shall be approved by the Department. Tanks shall be approved as follows: The tank design shall be approved based on the plans and specifications submitted in accordance with Subparagraphs (c)(1) through (c)(8) of this Rule.

- (1) The tank design shall be approved based on the plans and specifications submitted in accordance with Subparagraphs (c)(1) through (c)(8) of this Rule. After the tank design has been approved, a temporary identification number shall be assigned for tracking purposes.
- (2) The tank shall pass a structural load test as described in Subparagraph (c)(9) of this Rule. The test shall be performed and certified by a third party. The test shall be observed in person by the Department, LHD, PE, or a third party testing organization. If the tank passes the structural load test, then the tank shall be assigned a permanent identification number. Tanks shall not be sold for use in a wastewater system without a permanent identification number.
- (3) The structural design verification shall be required for new tanks, modifications to tank design, and when tank forms are sold to a different tank manufacturer.
- (4) Pump tanks may be tested and approved with a baffle wall, without a baffle wall, or with a partial baffle wall. The most limiting design produced by the manufacturer shall be tested.

(b) The tank manufacturer shall submit three copies of the plans and specifications for the initial design of each tank to the Department for approval.

#### (c) Plans and specifications for tanks with a total liquid capacity less than or equal to 4,000 gallons shall include the following:

all tank dimensions in inches, including:

(1)

- (A) top, bottom, and sidewall thickness and variations;
- (B) minimum and maximum dimensions on tanks with tapered or ribbed walls;
- (C) baffle wall location and minimum and maximum thickness and variations;
- (D) location and dimension of all openings in baffle wall for gas and liquid movement; and
- (E) dimensions of all compartments;
- (2) material type and strength, including reinforcement material and location, as applicable, specified by the manufacturer;
- (3) method for fastening the baffle wall to the tank interior;
- (4) liquid depth and operating capacity in gallons;
- (5) pipe penetration boot locations and pipe penetration boots approved in accordance with Rule .1404 of this Section;
- (6) methods and material for sealing sections and forming watertight joints in tanks with multiple sections;

- (7) drawings showing access openings, tank lids, access manhole risers, and other proposed appurtenances to the tank;
- (8) tank manufacturer and PE requirements for installation, including bedding, additional sealing methods, and leak testing procedures; and procedures.
- (9) documentation of proof of design. The tank shall withstand a minimum uniform live load of 150 pounds per square foot in addition to the dead weight of the material and all geostatic and hydrostatic loads to which an underground tank is normally subjected, such as active soil pressure on tank walls and the uplifting force of groundwater. The documentation shall be one of the following:
  - (A) a vacuum test of 4.24 inches of mercury held for five minutes meeting the following criteria:
    - (i) no loss in vacuum greater than two fifths of an inch of mercury during the test;
    - (ii) no deformation or deflection greater than two percent along any dimension unless shown by measurement or calculation to result in a reduction in volume no greater than two percent;
    - (iii) no distortion of the access openings occurs during the testing that prevents removal and replacement of the access opening lids at the conclusion of the test; and
    - (iv) for tanks constructed with integral risers, no distortion of the riser during the testing and the riser lid can be removed and replaced at the conclusion of the test;
  - (B) calculations from a PE that the tank can withstand the loading requirements of this Subparagraph and the performance requirements of Part (A) of this Subparagraph shall be met; or
  - (C) the tank shall be either IAPMO/ANSI Z1000 or CSA B66 certified and the tank manufacturer enrolled in a third party quality assurance and quality control program, which includes material testing and unannounced annual manufacturing facility audits.

(d) Plans and specifications for tanks with a total liquid capacity greater than 4,000 gallons and all tanks designed for traffic loads shall be designed by a PE in accordance with ASTM C890. Plans shall show the design, including all the information listed in Paragraph (c) of this Rule and engineering calculations showing the minimum and maximum soil burial depth, water table, and traffic load the tank is designed to support.

(e) Plans for tanks not proposed for general use and issued an identification number under this Section shall meet the minimum requirements of this Section and shall be approved by the Department.

(f) The Department or LHD may inspect approved tanks at the place of manufacture, the inventoried sites of the distributors, or at the installation of the tank in a wastewater system for compliance with the approved plans and specifications.

(g) Tanks found to be out of compliance shall be brought back into compliance by the tank manufacturer or the installer as directed by the Department or LHD. Tanks that are not or cannot be brought into compliance shall not be used in a wastewater system and the imprints identified in Rule .1402(d)(15) or (e)(8) of this Section shall be permanently marked over by the authorized agent.

History Note: Authority G.S. 130A-335(e), (f), and (f1); <u>S.L. 2024-49, s.4.42;</u> Eff. January 1, 2024.

# 15A NCAC 18E .1402 TANK DESIGN AND CONSTRUCTION

(a) Tanks shall be watertight, structurally sound, and not subject to corrosion or decay.

(b) Septic tanks and grease tanks shall have effluent filters and access devices approved in accordance with Rule .1404 of this Section. An effluent filter and support case shall be installed level in the outlet end of the septic tank or grease tank and shall meet the following criteria:

- (1) solvent welded to a minimum of three-inch PVC Schedule 40 outlet pipe;
- (2) be installed in accordance with filter manufacturer's specifications and effluent filter approval; and
- (3) be accessible and removable without entering the septic tank or grease tank.

(c) Septic tanks installed where the access openings on the top of the tank are deeper than six inches below finished grade shall have an access riser over each compartment with a cover that extends to within six inches of the finished grade. The opening of the access riser shall be large enough to accommodate the removal of the septic tank lid. When the top of the septic tank or access riser is below the finished grade, the location of the tank shall be visible at finished grade. When access risers are used they shall be installed in accordance with the Rules of this Subchapter, the manufacturer's specifications, and the Department's approval.

(d) Septic tanks shall meet the following minimum design standards:

- (1) a minimum liquid depth of 36 inches;
- (2) a minimum of nine inches freeboard, measured as the air space between the top of the liquid and the bottom of the tank top. Venting of the tank shall be provided to prevent the buildup of gases;
- (3) the approved septic tank capacity shall be determined as the liquid volume below the outlet invert to the bottom of the tank;
- (4) the length of the tank shall be a minimum of twice as long as the width, as measured by the longest axis and widest axis based on the internal tank dimensions;
- (5) there shall be three inlet openings in the tank, one on the tank end and one on each sidewall of the inlet end of the tank;
- (6) outlet openings shall have a cast or manufactured penetration point and include a watertight, sealed, non-corrodible, and flexible connective sleeve. A flexible connective sleeve shall be able to bend without breaking. The connective sleeve shall meet ASTM C1644 for precast concrete tanks or ASTM C1644, C923, or C564 for thermoplastic or glass-

fiber-reinforced polyester tanks and be approved by the Department if it meets the requirements of this Subparagraph and Rule .1404 of this Section;

- (7) inlet penetrations shall be greater than or equal to four inches in diameter and outlet penetrations shall be greater than or equal to three inches in diameter;
- (8) there shall be no openings below the septic tank operating liquid level;
- (9) the outlet shall be through an effluent filter approved in accordance with Rule .1404 of this Section, and secured in place in an effluent filter support case. The effluent filter case inlet shall extend down to between 25 and 50 percent of the liquid depth measured from the top of the liquid level. Other methods of supporting the effluent filter case and for making pipe penetrations shall be approved by the Department on a case-by-case basis upon a showing that the performance is identical to those designed in accordance with this Rule;
- (10) the invert of the outlet shall be a minimum of two inches lower in elevation than the invert of the inlet;
- (11) all septic tanks shall be designed with a partition so that the tank contains two compartments. The following conditions shall be met:
  - (A) the partition shall be located at a point not less than two-thirds or more than three-fourths the length of the tank from the inlet end;
  - (B) the partition shall be designed, manufactured, installed, and maintained to remain in position when subjected to a liquid capacity in one compartment that corresponds with the lowermost elevation of the water passage slot or holes;
  - (C) the partition shall be designed to create a gas passage, not less than the area of the inlet pipe, and the passage shall not extend lower than seven inches from the bottom side of the tank top;
  - (D) the top and bottom sections of the partition shall be designed to create a water passage slot four inches high for the full interior width of the tank, or a minimum of two four- or five-inch openings, or one four- or five-inch opening per 30 horizontal linear inches of baffle wall, whichever is greater, may be designed into the partition instead of the four-inch slot;
  - (E) the partition shall be designed, manufactured, and installed to create an average opening not greater than onehalf inch between the partition and the tank wall below the liquid level, with a tolerance of one-half inch;
  - (F) the entire liquid passage in the partition wall shall be located between 25 and 50 percent of the liquid depth of the tank, as measured from the top of the liquid level; and
  - (G) other methods for designing the partition shall be approved by the Department on a case-by-case basis upon a showing that the performance is identical to those designed in accordance with this Rule;
- (12) access openings shall be provided in the top of the tank, located over each compartment, and have a minimum opening of 15 inches by 15 inches or 17 inches in diameter. The opening shall allow for maintenance and removal of internal devices of the septic tank;
- (13) access risers and covers shall be designed and manufactured to prevent surface water infiltration;
- (14) tank lids and riser covers shall be locked, secured with fasteners, or weigh a minimum of 40 pounds, but no more than 80 pounds; and
- (15) all septic tanks shall bear an imprint or embossment identifying the manufacturer, the septic tank serial number assigned to the manufacturer's plans and specifications approved by the Department, and the liquid or working capacity of the tanks. The imprint or embossment shall be located to the right of the blockout made for the outlet pipe on the top or end of outlet end of the tank.
- (e) Pump tanks shall meet the design requirements of Paragraph (d) of this Rule with the following modifications:
  - (1) a watertight access riser with removable cover shall be located over the pump. The access riser shall extend to a minimum of six inches above finished grade and shall be designed and maintained to prevent surface water infiltration;
  - (2) the access opening over the pump shall have a minimum opening of 24 inches in diameter or equidimensional opening;
  - (3) when two or more pumps are required in accordance with Rule .1101(b) of this Subchapter the access openings shall be sized to allow for pump removal, operation, and maintenance;
  - (4) tanks may be designed with a single compartment. If a partition is provided, the partition shall be designed to contain a minimum of two four-inch diameter circular openings, or openings with an equivalent area, located no more than 12 inches above the tank bottom;
  - (5) there shall be no requirement as to tank length, width, or shape, provided the tank satisfies all other requirements of the rules of this Section;
  - (6) the invert of the inlet openings shall be located within 12 inches of the tank top. No freeboard shall be required in the pump tank;
  - (7) tanks shall be vented if located more than 50 feet from the facility, and accessible for routine maintenance;
  - (8) all pump tanks shall bear an imprint or embossment identifying the manufacturer, the pump tank serial number assigned to the manufacturer's plans and specifications by the Department, and the liquid or working capacity of the tank. The imprint or embossment shall be located to the left of the blockout made for the outlet pipe on the top or end of outlet end of the tank; and
  - (9) the pump tank working capacity shall be the entire internal tank volume.
- (f) Grease tanks shall be septic tanks approved in accordance with Paragraph (d) of this Rule with the following modifications:

- (1) the liquid passage between chambers shall be located between 40 and 60 percent of the operating liquid depth measured from the top of the liquid level. The liquid passage between chambers may be made using a sanitary tee extending down between 40 and 60 percent of the liquid depth measured from the top of the liquid level;
- (2) when sanitary tees are used as the liquid passage through an interior compartment partition, an access opening and riser to grade over the tees shall be provided for servicing and routine maintenance;
- (3) when two or more tanks are used in series, a sanitary tee shall be provided in the outlet end of each interconnected tank extending down between 40 and 60 percent of the liquid depth;
- (4) the final chamber shall contain an effluent filter and support case extending down between 40 and 60 percent of the liquid depth. The effluent filter shall be approved by the Department for use in grease tanks. The grease rated effluent filter shall be sized for the DDF and have openings of 1/32-inch or less; and
- (5) access risers shall extend to finished grade and be capped with cast iron manhole rings and covers. Lockable aluminum hatches may be substituted for cast iron manhole rings and covers in non-traffic areas. Aluminum hatches or manhole rings and covers shall be designed and maintained to prevent surface water infiltration. Locks shall be the responsibility of the person owning or controlling the system.
- (g) Siphon tanks shall meet the design requirements of Paragraph (e) of this Rule and shall:
  - (1) be designed in accordance with the requirements of this Rule and Rule .0804 of this Subchapter;
  - (2) provide three inches of freeboard;
  - (3) have the invert of the inlet pipe three inches above the siphon trip level; and
  - (4) have a watertight access opening over each siphon with an opening of 24 inches, extending to finished grade, and designed to prevent surface water inflow.

History Note: Authority G.S. 130A-335(e), (f), and (f1); 130A-335.1; <u>S.L. 2024-49, s.4.43;</u> Eff. January 1, 2024.

# 15A NCAC 18E .1403 TANK MATERIAL REQUIREMENTS

(a) Tanks approved in accordance with this Section shall be constructed of materials capable of resisting corrosion from sewage and sewage gases, structurally sound, and watertight.

(b) Reinforced precast concrete tanks shall meet the following minimum material and construction requirements:

- (1) the ends and sides of the tank shall have a minimum thickness of two and one-half inches. The top and bottom of the tanks shall be a minimum of three inches thick;
- (2) the top, bottom, end and sides of the concrete tank and tank lid shall be reinforced by using a minimum reinforcing of six-inch by six-inch No. 10 gage welded steel reinforcing wire. Reinforcement shall be placed to maximize the structural integrity of the tank;
- (3) alternative reinforcement designs may be used when they perform in a manner equal to or more effective than the reinforcement design described in Subparagraph (2) of this Paragraph;
- (4) when the concrete tank, tank lid, riser, or riser cover are subjected to vehicular traffic, the tank shall be designed by a PE to handle the traffic load in accordance with ASTM C890;
- (5) any tank installed deeper than three feet shall be designed by a PE for the proposed tank burial depth. The tank design shall be submitted to the Department for review. The design shall be approved when documentation is provided to show that the proposed tank design can withstand all active and passive loads on the tank, including the additional soil weight from a deeper burial depth.
- (6) the concrete shall achieve a minimum 28-day compressive strength of 4,000 psi. The concrete shall meet a compressive strength of 3,500 psi prior to removal of the tank from the place of manufacture. It shall be the responsibility of the manufacturer to certify that the tank meets this condition; The delivery of the tank to the jobsite is considered the certification by the tank manufacturer of these conditions. An authorized agent, PE, or AOWE shall follow the requirements of this Rule on any subsequent testing to verify this certification. Readings from a rebound hammer shall not be used to reject a tank at the jobsite. The LHD, PE, or AOWE shall report test readings taken in accordance with this Rule to the On-Site Water Protection Branch and shall be used to schedule a future random inventory verification by the Department;
- (7) <u>Rebound hammers shall be used in accordance with the following when checking the strength of a precast concrete</u> tank:
  - (A) the surface of the concrete tank tested should be smooth, dry, and free of honeycombing:
  - (B) the concrete to be tested must be at least four inches thick;
  - (C) the readings should be taken around the edge of the tank;
  - (D) tests should not be on trowelled surfaces; and

(8)

(E) the test locations should be at least one inch from the edge of the tank or the location of another test point;

The procedure for testing a tank with a rebound hammer shall be as follows:

- (A) take a total of 12 readings from around the tank;
- (B) the rebound hammer shall be directly perpendicular to the surface of the tank;
- (C) the readings should be from different sides of the concrete tank;
- (D) take readings from at least two different edges on each tank side, if possible;
- (E) eliminate the highest and lowest readings taken;

- (F) average the remaining ten readings and use that average to determine the concrete compressive strength from the conversion chart on the rebound hammer;
- (G) adjust the concrete compressive strength by plus or minus 25 percent to account for the rebound hammer's 25 percent error in the reading; and
- (H) the rebound hammer should be calibrated annually.
- (7)(9) tanks manufactured in multiple sections shall be joined and sealed at the joint by using butyl rubber or other pliable sealant meeting ASTM C990 or other material that has been approved by the Department when documentation has been provided to show that the material meets all performance requirements of ASTM C990. Documentation shall also be provided to the Department to show that the material is waterproof and corrosion resistant; and
- (8)(10) tank lids and riser covers shall have a durable handle made of corrosion-resistant materials and capable of pull capacity sufficient for the weight of the lid or cover.

(c) Thermoplastic tank materials shall conform with IAPMO/ANSI Z1000 or CSA B66 requirements.

- (d) Glass-fiber-reinforced polyester tanks shall meet the following requirements:
  - (1) top, bottom, ends, and sides of the tank shall have a minimum thickness of one-fifth inches. The baffle wall shall be a minimum of 3/16-inches thick;
  - (2) material and laminate requirements specified in IAPMO/ANSI Z1000 or CSA B66 for glass-fiber-reinforced polyester tanks; and
  - (3) enrolled in a third-party quality assurance and quality control program, which include material testing and unannounced annual audits.

(e) Cast or manufactured in place tanks shall be designed by a PE, if required by G.S. 89C, and approved by the Department when the tank design, construction, and materials meet the criteria set forth in this Rule and Rule .1402 of this Section.

History Note: Authority G.S. 130A-335(e), (f), and (f1); <u>S.L. 2024-49, s.4.44;</u> Eff. January 1, 2024.

# 15A NCAC 18E .1404 PLANS AND SPECIFICATIONS FOR RISERS, EFFLUENT FILTERS, AND PIPE PENETRATION BOOTS

(a) All <u>plastic or fiberglass</u> risers, effluent filters, and pipe penetration boots proposed for use in a wastewater system shall be approved by the Department prior to being offered for sale or use in North Carolina.

(b) Three copies of the plans and specifications for the initial design of each <u>plastic or fiberglass</u> riser, effluent filter, or pipe penetration boot shall be submitted to the Department. Plans for <u>plastic or fiberglass</u> risers, effluent filters, and pipe penetration boots shall be approved by the Department and an approval letter issued when the design is found to comply with this Section. All changes or modifications to <u>plastic or fiberglass</u> risers, effluent filters, or pipe penetration boots shall be approved by the Department when the changes or modifications comply with the requirements of this Rule.

(c) <u>Risers Plastic or fiberglass risers</u> and riser lids shall be able to withstand a minimum uniform live loading of 300 pounds per square foot or a minimum 1,500 pound load applied in a 10 inch by 10 inch area centered on the lid, in addition to all loads to which a riser is normally subjected, such as dead weight of the material and soil cover and active soil pressure on riser walls.

(d) Riser Plastic or fiberglass riser plans and specifications submitted to the Department for review and approval shall show the design of the riser and include the following information:

- (1) manufacturer's name, mailing address, phone and fax numbers, email address, and name of manufacturer's point of contact;
- (2) physical dimensions of the riser and riser cover, including wall thickness, internal diameter, proposed casting or installation details and methods, and pipe penetrations;
- (3) material type and strength, including reinforcement material and location as required;
- (4) documentation from a third-party showing that the riser meets the load requirements specified in Paragraph (c) of this Rule;
- (5) plans for septic tank risers of a secondary lid, concrete plug, or other safety device that shall be provided inside the riser for security and to prevent accidental entry;
- (6) plans for pump tank risers of primary and secondary safety mechanisms that shall be provided with the riser. The primary safety mechanism shall be a locking riser lid, ring and lock, or other riser lid locking or tamper-resistant mechanism. The secondary safety mechanism shall be a secondary lid, concrete plug, or other safety device to be provided inside the pump tank riser; and
- (7) specifications for application, installation, operation, and maintenance for both new and retrofit applications for single and multiple riser sections.

(e) Effluent filter plans and specifications submitted to the Department for review and approval shall show the design of the effluent filter and include the following information:

- (1) manufacturer's name, address, phone and fax numbers, and contact name;
- (2) documentation and a written statement from the manufacturer that the effluent filter is designed, constructed, and performs in compliance with G.S. 130A-335.1(a);
- (3) capacity and wastewater strength for all models of proposed filters to be approved; and
- (4) specifications for application, installation, operation, and maintenance.

(f) Pipe penetration boot plans and specifications submitted to the Department for review and approval shall show the design of the pipe penetration boot and include the following information:

- (1) manufacturer's name, address, phone and fax numbers, and contact name;
- (2) design specifications and materials used in the manufacture of pipe penetration boot components;
- (3) applicable testing results from third-party verification showing pull and flexibility testing;
- (4) documentation of a watertight seal around the piping and any component or device needed to ensure the seal, such as non-corrodible adjustable bands;
- (5) documentation that the pipe penetration boot meets the requirements of ASTM C1644 for precast concrete tanks or ASTM C1644, C923, or C564 for thermoplastic or glass-fiber-reinforced polyester tanks; and
- (6) specifications for application, installation, operation, and maintenance of the pipe penetration boot.

(g) Plans for prefabricated plastic or fiberglass risers, effluent filters, and pipe penetration boots, other than those approved for general use and issued an approval letter under this Rule, shall be considered for approval on a case-by-case basis. The plastic or fiberglass riser, effluent filter, or pipe penetration boot shall be approved if it is determined that it meets the requirements of this Rule based on information provided by the manufacturer to the Department.

(h) Concrete risers and riser lids shall be able to withstand a minimum uniform live loading of 300 pounds per square foot or a minimum 1,500 pound load applied in a 10 inch by 10 inch area centered on the lid, in addition to all loads to which a riser is normally subjected, such as dead weight of the material, soil cover, and active soil pressure on riser walls. Concrete risers shall meet the following requirements:

- (1) septic tank risers shall have a secondary lid, concrete plug, or other safety device that shall be provided inside the riser for security and to prevent accidental entry; and
- (2) pump tank risers shall have a secondary safety mechanism that shall be provided with the riser. The secondary safety mechanism shall be a secondary lid, concrete plug, or other safety device to be provided inside the pump tank riser.

History Note: Authority G.S. 130A-335(e), (f), and (f1); 130A-335.1; <u>S.L. 2024-49, s.4.45;</u> Eff. January 1, 2024.

# 15A NCAC 18E .1405 RISERS, EFFLUENT FILTERS, AND PIPE PENETRATION BOOTS APPROVAL RENEWAL

(a) All <u>plastic or fiberglass</u> riser, effluent filter, and pipe penetration boot approvals shall expire on December 31 of each year. Riser, <u>Plastic or fiberglass riser</u>, effluent filter, and pipe penetration boot manufacturers who wish to continue product approval shall submit annually a proprietary product renewal form provided by the Department no later than November 30 of each year.

(b) The approval renewal form shall include the following elements:

- (1) manufacturer's name, mailing address, phone and fax numbers, email address, , and manufacturer's point of contact;
- (2) model number(s) approved; and
- (3) a notarized statement that the product has not changed from the previous year without prior approval from the Department.

(c) The Department shall notify the manufacturer of the pending <u>plastic or fiberglass</u> riser, effluent filter, and pipe penetration boot Approval expiration in writing no later than September 30 of each year. The notification shall include information on how to request <u>plastic or fiberglass</u> riser, effluent filter, and pipe penetration boot renewal.

(d) The <u>plastic or fiberglass</u> riser, effluent filter, and pipe penetration boot approval shall be deemed renewed upon receipt of a renewal form that contains all of the elements set out in Paragraph (b) of this Rule.

*History Note: Authority G.S. 130A-335(e) and (f); 130A-343; <u>S.L. 2024-49, s.4.46;</u> <i>Eff. January 1, 2024.* 

# SECTION .1700 – APPROVAL AND PERMITTING OF WASTEWATER SYSTEMS, TECHNOLOGIES, COMPONENTS, OR DEVICES

#### 15A NCAC 18E .1713 LOCAL HEALTH DEPARTMENT RESPONSIBILITIES

To implement this Section the LHD shall:

- (1) When a Provisional System is proposed, confirm that the designated repair system complies with the provisions of Rule .0508 of this Subchapter and with individual PIA Approval requirements, except:
  - (a) when an existing wastewater system is available for immediate use, including connection to a public or community wastewater system;
  - (b) when the Provisional System is used as a repair to an existing malfunctioning system when there are no other approved Innovative or Accepted repair options; or
  - (c) as provided in G.S. 130A-343(f) for Provisional Systems.
- (2) Notify the Department of all IPs, CAs, and OPs issued for Provisional Systems.
- (3) Notify the Department of all OPs issued for Innovative Systems.
- (4) Permit systems designated as Accepted Systems Systems, without requiring a request from the owner, in an equivalent manner to a conventional system. system at the owner's request. The Accepted System shall be sited and sized in accordance with Section .0900 of this Subchapter or PIA Approval. The type of Accepted System installed shall be

indicated on the OP. The owner shall re-apply to the LHD and receive a new or revised IP or CA for any of the following before system installation:

- (a) location of any part of the dispersal field outside of the approved initial dispersal field area;
- (b) changes to the trench depth, and slope correction if applicable, specified on the IP or CA;
- (c) changes to the effluent distribution method; or
- (d) changes to the DDF or wastewater strength.
- (5) Grant permit reductions in total trench length less than or equal to 25 percent for Innovative or Accepted Systems only to dispersal fields receiving DSE or better quality. A facility with a full kitchen shall not be granted a permit reduction in total trench length.
- (6) Grant facilities generating HSE the 25 percent reduction allowed for Innovative or Accepted Systems if the system includes an approved advanced pretreatment system designed to ensure effluent strength equal to or better than DSE.
- (7) Prohibit issuance of an OP for a proprietary system installed by a person not authorized by the manufacturer, unless the manufacturer of the proprietary system approves the installation in writing.
- (8) Inform the Department, as well as the manufacturer or their authorized representative, of any system determined to be malfunctioning. If the system has been permitted in accordance with G.S. 130A-336.1 or G.S. 130A-336.2 and Rule .0207 of this Subchapter, the LHD shall instruct the owner to contact the PE or AOWE for determination of the reason and the malfunction and development of an NOI for repairs.
- (9) Issue a NOV to the owner when the system is determined to be malfunctioning in accordance with Rule .1303(a)(1) and (2) of this Subchapter or when an individual advanced pretreatment system at a single site is out of compliance in accordance with Rule .1302(f) of this Subchapter. The notice shall identify the violations and steps necessary to remedy the problems, including modification of the system, established time frame to achieve compliance, other follow-up requirements, and specify further enforcement possibilities if compliance is not achieved.
- (10) Include in its monthly activity report submitted to the Department the following information identified by unique codes:
  - (a) number of new system OPs issued for PIA Systems;
  - (b) number of new system OPs issued for Accepted Systems;
  - (c) number of CAs issued for Provisional Systems, including system type;
  - (d) number of CAs issued for repairs of PIA Systems, including system type being repaired;
  - (e) number of CAs issued for repairs of Accepted Systems, including system type being repaired; and
  - (f) repair system type.

History Note: Authority G.S. 130A-335(e) and (f); 130A-343; <u>S.L. 2023-77, s.21; S.L. 2024-49, s.4.47;</u> Eff. January 1, 2024.